

Water Supply Assessment for Barber Creek 06 Barrow County, Georgia



Prepared for:
**Georgia State Soil and Water Conservation
Commission**

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EXECUTIVE SUMMARY

The Georgia Soil and Water Conservation Commission (GSWCC), in partnership with the Natural Resources Conservation Service (NRCS) and the Georgia Environmental Protection Division (EPD) initiated a study to evaluate whether or not any of the existing watershed dams, designed and constructed under federal laws PL 544 and PL 566, could be modified to serve as water supply reservoirs. The evaluation process went through several iterations, the most recent of which can be found in the Finding Report dated December, 2007 on file with the GSWCC. The Finding Report identified 20 structures that had sufficient potential for relatively high yields with relatively small environmental and infrastructural impacts, when compared to the other projects evaluated. The selected twenty dams were further evaluated to identify project parameters. Eight additional structures were selected in 2008 for further evaluation.

The following report summarizes the evaluation of the Barber Creek Structure Number 6, which is located in Barrow County, Georgia. For the purposes of this report, the existing normal pool will be raised to impound a water supply pool having a surface area of approximately 510 acres.

For convenience, the following summary lists the major findings of this evaluation. This summary should not be utilized as a separate document or in lieu of reading the entire report, including the Appendix.

- Approximately 787 acres of land will be impacted by the proposed reservoir and dam raising
- Approximately 17 structures will be impacted by the proposed reservoir and dam raising
- Eight county roads will be impacted.
- Approximately 1,000 feet of natural gas pipeline will be affected.
- Approximately 6,500 feet of telephone transmission lines will be affected.
- For the modeled conditions, the drought of record in the Barber Creek 6 basin the current drought. For a water supply storage of approximately 3,375 million gallons and supplementation of natural reservoir inflow by pumped diversions (maximum 20 million gallons per day, mgd) from the nearby Middle Oconee River, the safe yield of the reservoir is estimated to be 4.9 mgd.
- Approximately 35 acres of palustrine wetlands will be impacted by the proposed reservoir and dam raising
- Approximately 45 acres of lacustrine/palustrine open waters will be impacted by the proposed reservoir and dam raising
- Approximately 20,627 linear feet of lower perennial streams will be impacted by the proposed reservoir and dam raising
- Approximately 18,716 linear feet of intermittent streams will be impacted by the proposed reservoir and dam raising
- Review of existing cultural resources information indicated one identified cultural resource site, a Watershed Lake Dam within the maximum reservoir pool limits of Barber Creek 6.
- Review of available information did not indicate any primary or secondary trout streams or 303(d) / 305(b) listed streams occurring within the maximum reservoir pool limits of Barber Creek No. 6.
- Review of existing threatened and endangered species information identified three federally and state protected species documented from Barrow County, Georgia
- Project cost is estimated in 2008 dollars at \$90,000,000.

PREFACE

The results of the analyses presented herein are based upon United States Geological Survey (USGS) quadrangle maps and, therefore, should be utilized for planning purposes only. If the subject project is identified as having a possibility of progressing past this analysis, additional studies will be required. These studies will include but not be limited to detailed environmental evaluations, detailed yield analyses, preliminary engineering design, and detailed cost estimating. These additional studies will be required prior to beginning detailed design work and/or land acquisition. The level of study presented herein shall be considered as a screening tool to evaluate the proposed project relative to other projects. Until further studies are performed, actual yield and costs associated with the entire project cannot be readily determined.

INTRODUCTION

The project team of Schnabel Engineering South, LLC (Schnabel) and Jordan Jones and Golding (JJ&G) were retained by the Georgia State Investment and Financing Commission as the agent for the Georgia Soil and Water Conservation Commission to evaluate 166 existing flood control structures. The subject structures were originally designed and constructed under Federal laws PL 544 and PL 566 to control storm water runoff (flooding) and collect sediment. The goal of this evaluation was to identify impoundments that could be enlarged to provide a relatively reliable water supply. The results of the evaluation were utilized to select twenty of the dams and reservoirs that had potential for relatively high yields with relatively small environmental and infrastructural impacts, when compared to the other projects evaluated. The selected twenty dams were further evaluated to identify project parameters. An additional eight structures were selected for further evaluation. The additional evaluation included the following:

- More detailed yield analyses
- More detailed environmental evaluation
- Cost estimation of proposed modifications

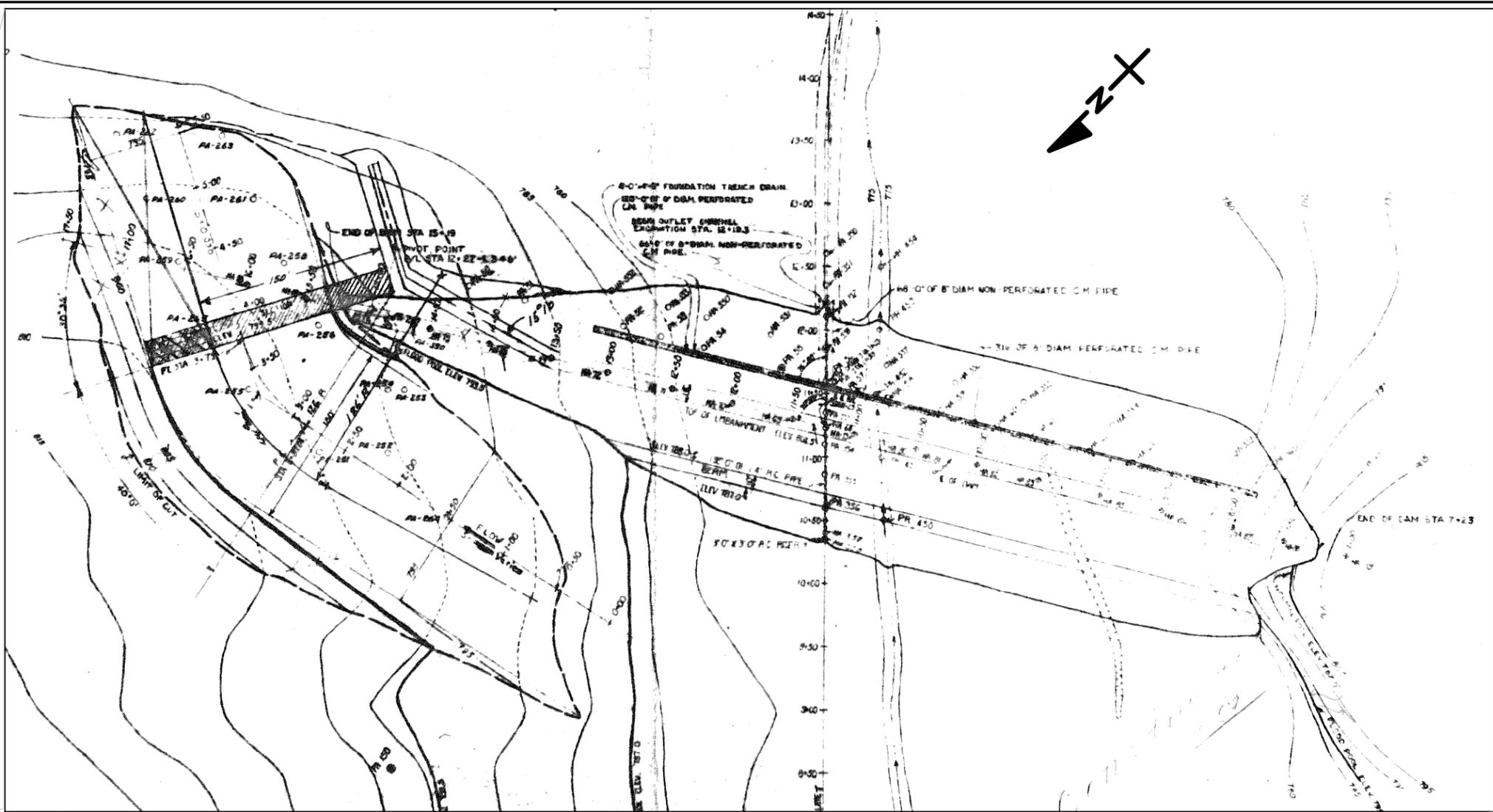
The Barber Creek Watershed Dam Number 6 in Barrow County, Georgia was one of the structures selected for further evaluation.

BACKGROUND

The subject dam, Barber Creek Watershed Dam Number 6 (Barber Creek Dam No. 6), is located approximately 1.5 miles southwest of Statham, Georgia in Barrow County.

The existing dam was designed in 1959 rehabilitated in 1984. As designed, the dam had a crest elevation of 802.5 feet and impounded a reservoir that had a surface area of approximately 52.8 acres at an approximate normal pool elevation of 788 feet. The crest of the emergency spillway was designed to be at elevation 799.5 feet. Figure 1 shows the location of the subject dam within the county as well as a plan view of the existing embankment and emergency spillway.

According to the Soil Conservation Service (SCS), now known as the Natural Resources Conservation Service (NRCS), Dam Inventory sheet, the dam was originally designed and constructed as a Class 'A' or low-hazard dam. The state Safe Dams program has classified the dam as a Category I, or high-hazard structure. When designed, the emergency spillway (now referred to as an auxiliary spillway) had a four percent chance of operating in any given year. This results in the auxiliary spillway operating during storm events equal to and greater than the 25-year event. With the exception of engineering, land acquisition, and project administration, the dam was completed for a cost of approximately \$34,785.



Needs and Demand Evaluation

Population projections for Barrow County through the year 2030 were obtained from the Barrow County Comprehensive Plan (published in 2007), while projections from 2030 to 2050 were obtained from the Northeast Georgia Water Resources Study Update (2004). In years where projections were not available, population was interpolated from published data. Projections outside of 2050 were extrapolated based on the average growth rate that was shown through 2050. These projections can be seen in Table 1.

Table 1
Population Projection

Year	Population Projection
2000	46,144
2005	59,954
2010	80,000
2015	102,000
2020	124,370
2025	169,493
2030	173,750
2035*	196,640
2040	219,530
2045*	239,275
2050	259,020
2055*	279,500
2057*	287,692

Data Source: 2005-2030 from Barrow County Comprehensive Plan, 2040-2050 from NE Georgia Water Resources Study Update

**Population Calculated based on yearly % growth from published data*

Water demand projections were calculated based on population projections and water withdrawal data for Barrow County in 2000. According to the US Census, the population of Barrow County was 46,144 in 2000, while the water withdrawal was 6.8 million gallons per day (MGD) based on the document “Water Use in Georgia by County for 2000”, (Information Circular 106, Julia Fanning, USGS, Atlanta, 2003). The Barrow County Water and Sewerage Authority currently holds a surface water withdrawal permit from the Mulberry River for 5.1 MGD. Additionally, the City of Statham holds a surface water permit for 0.8 MGD (numbers are reported in permitted monthly average).

The overall usage was calculated to be 147 gallons per day (gpd) per person. This number was used as a constant through 2057 to create water withdrawal projections. The water withdrawal projection for 2057 was calculated to be approximately 42.3 MGD. This figure includes all unaccounted for water (UAW), and the assumption that industrial usage would increase with the increase in Barrow County population. Water withdrawal projections are shown in Table 2.

Table 2
Water Withdrawal Projection

Year	Water Withdrawal Projection (MGD)
2000	6.8
2005	8.8
2010	11.8
2015	15.0
2020	18.3
2025	24.9
2030	25.5
2035	28.9
2040	32.3
2045	35.2
2050	38.1
2055	41.1
2057	42.3

Proximity to Surface Water Intakes

There is a surface water intake in Barber Creek 06. It is operated by the City of Statham.

ENGINEERING FACTORS

Proposed Dam

The proposed dam, which will incorporate the existing dam, will have a crest elevation of 840 feet, an auxiliary spillway elevation of 830 feet, and a water supply pool elevation of 828 feet. The proposed dam will impound a reservoir that has a surface area of approximately 510 acres and storage volume of approximately 3,375 million gallons (MBG) at the water supply pool elevation. A plan view of the proposed reservoir is shown in Figure 2.

Several engineering assumptions were made pertaining to spillway configuration. The spillway system for the proposed dam was assumed to consist of a principal spillway in the form of a 2' by 6' interior dimension reinforced concrete riser with a 24-inch diameter reinforced concrete low-level outlet pipe and an auxiliary spillway in the form of a 260-foot wide reinforced concrete chute spillway with ogee crest. The intent of the proposed principal spillway is to approximate the flows that are being discharged by the current spillway system during the two through 100-year storm events. The size of the auxiliary spillway was approximated by estimating the peak inflow that would occur during the Probable Maximum Precipitation (PMP) event and computing the spillway width that would be required to pass the estimated inflow with a given amount of hydraulic head. The available hydraulic head was determined by comparing the drainage basin area to lake surface area. The structures that had a drainage basin area to lake surface area ratio equal to or in excess of ten were allotted 15 feet of hydraulic head to pass the PMP inflows, while the structures that had a ratio of less than ten were allotted ten feet of hydraulic head to pass the PMP inflows. The assumption that the dam would be required to pass the inflow resulting from the PMP storm event is based on the history of the Georgia Department of Natural Resources Environmental Protection Division Safe Dams Program (Safe Dams) reviewing plans for water supply reservoir dams regardless of classification. As such, the dam would generally be required to comply with the engineering guidelines established by Safe Dams. The proposed dam would have a relatively high likelihood of being classified as high-hazard or Class 'C' by the NRCS, as well as Safe Dams. For this reason, it has been assumed that the dam will be required to pass the full PMP storm event.

The proposed dam and flood pool will:

- Impact 17 structures
- Require the purchase of 739 acres from 75 parcels
- Require the purchase of 48 acres of easement area for state required buffer
- Impact eight local/county roads

Figure 3 displays the proposed reservoir area as well as the buffer and affected parcels. The 17 affected structures were identified from aerial photographs. The types of structures were not identified on the ground and could be houses, barns, trailers, etc. A more detailed ground survey will be required to determine the type of each structure and the corresponding purchase price of each structure.

Figure 2
Proposed Reservoir Area Map

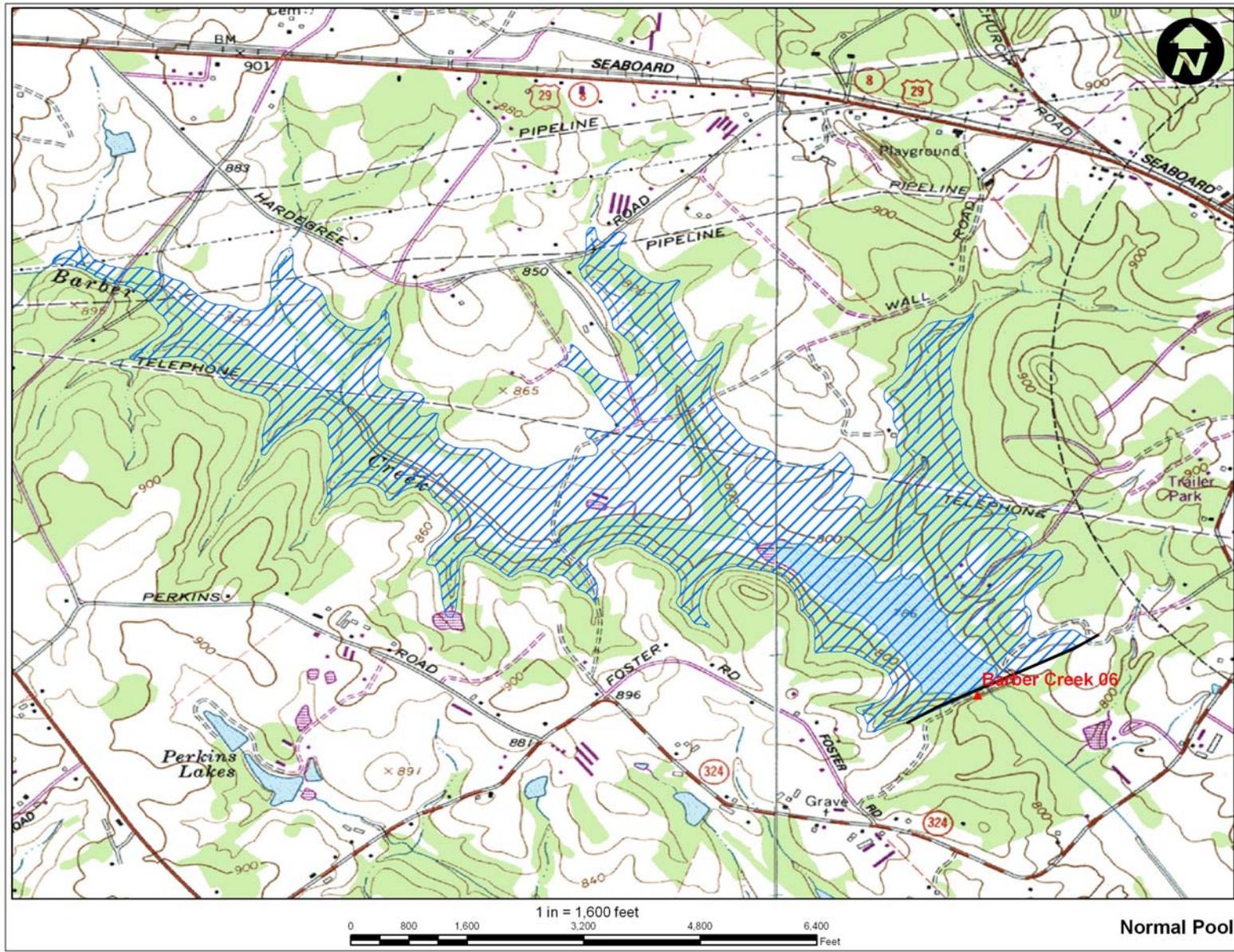
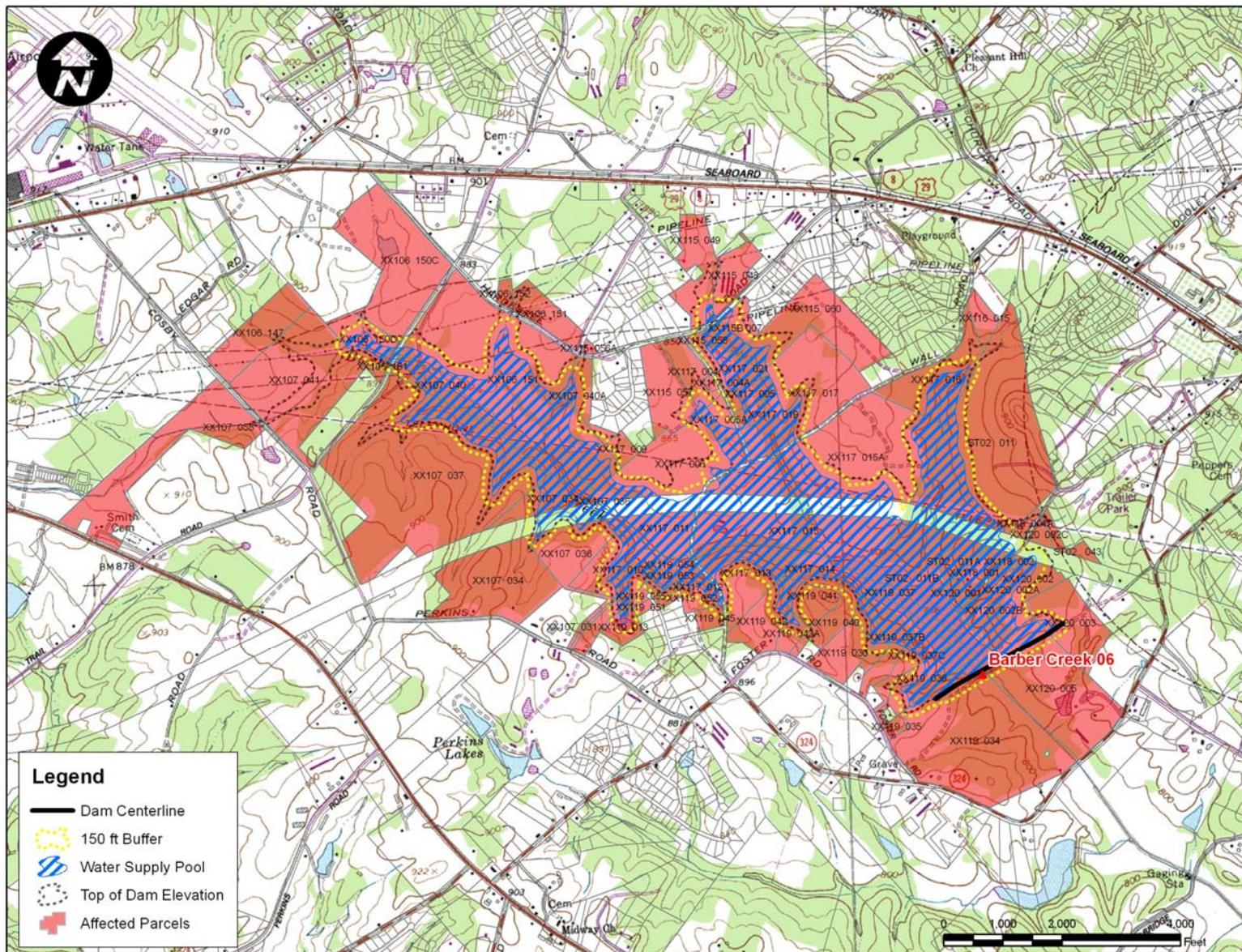


Figure 3
Land Acquisition and Buffer Areas



SAFE YIELD ANALYSIS

Definition

Reservoir safe yield is generally defined as the reliable withdrawal rate of water with acceptable quality that can be provided by reservoir storage through the critical drought period. The critical drought period in the State of Georgia is defined as the drought of record and in any given drainage basin can vary depending on reservoir size and other factors. The drought of record for the Oconee River basin is current drought; however the current drought does not yet exhibit recovery. Therefore the safe yield presented in this study was based on the current drought, extended with hypothetical flow data. Safe yield was simulated using a constant average annual demand. The justification for this is that while total water demands after declaration of a drought condition are usually less than normal, this situation is typically offset by higher than average demands prior to declaration of the drought condition. Safe yield is dependent upon the storage and hydrologic (rainfall/runoff/evaporation) characteristics of the source and source facilities, the selected critical drought, upstream and downstream permitted withdrawals, and the minimum in-stream flow (MIF) requirements.

The proposed reservoir is a “pumped-storage” reservoir, where natural inflow into the reservoir is supplemented with pumped diversions from a nearby larger stream or river. Water is pumped from a larger river when runoff is plentiful, and is stored in the reservoir for times of drought. Pumped diversions increase safe yield, and generally result in fewer environmental impacts compared with reservoirs on main-stem rivers.

Analysis Method

Two gages with a combined record period of 71 years (1937-present) were used to simulate flow for the safe yield analysis, and noted in the table below.

Table 3
Gage Summary

USGS Gage	Gage Name	Record Period	Drainage Area (mi²)
02217500	Middle Oconee River near Athens	10/01/1901 – 09/30/1902 01/01/1929 – 03/31/1932 05/01/1937 - Present	392
02217475	Middle Oconee River near Arcade	03/01/1987 -Present	332

Where data was available from both gages, preference was given to the Arcade gage (1987-present), given its location above (upstream of) withdrawals by the Upper Oconee Basin Authority and the City of Athens, and given its proximity to the proposed project. For the period between about 1980 (when Athens intake became operational) and 1987, simulated flows are likely less than actual streamflows due to Athens’ withdrawals; however, given that the mid-

1980's drought is not the drought of record, this has negligible effects on the analysis results. To adjust the Arcade data for more recent upstream withdrawals, we obtained withdrawal records from the City of Winder (Mulberry River) and added their withdrawals to the gage data. The combined record from the two Middle Oconee gages was then used to simulate streamflows in the safe yield study for the combined 71 year record period.

The combined gage record includes four major droughts (1954-56, 1986-88, 1999-2002, 2007 - present). Since there is insufficient data to model the full extent of the current drought, streamflow data was extended into the future (2009 – 2012) using data from 2001- 2004. Comparison of the 2008 flows have closely tracked those of 2000, thus extension of the flows beyond 2008 using the 2001-2004 data is considered reasonable. Therefore the estimated safe yield and pumping capacities presented in this study were based in part on the current drought, extended with hypothetical flow data.

The diversion pump station was assumed to be located below the confluence of Beech Creek with the Middle Oconee River. The straight line pipe distance between the dam and diversion location was estimated at 6.3 miles.

The following drainage areas were used in the analysis:

- Dam Site (Barber Creek): 5.84 mi²
- Diversion (Middle Oconee River): 333 mi²

The pumped diversion location and watershed are shown in Figure 4. The maximum pool level at top of dam was selected to limit the number of structures impacted by the proposed reservoir. From that level, a freeboard allowance of 10 feet between the top of dam and the auxiliary spillway was incorporated to pass the spillway design flood (assumed to be the probable maximum flood). Additional depth to maintain existing flood storage volume (1150 Ac-ft, or 375 MG) was subtracted from the auxiliary spillway elevation to compute the water supply pool elevation used in the analysis of safe yield. Note that more detailed topographic mapping would be needed to more closely approximate the safe yield of the proposed reservoir. Table 4 summarizes the various reservoir elevations and approximate storage volumes. Calculation of stage-area and stage-storage curves is presented as Figure A-1 in the Appendix. Figure 5 below is the stage-storage curve for the reservoir.

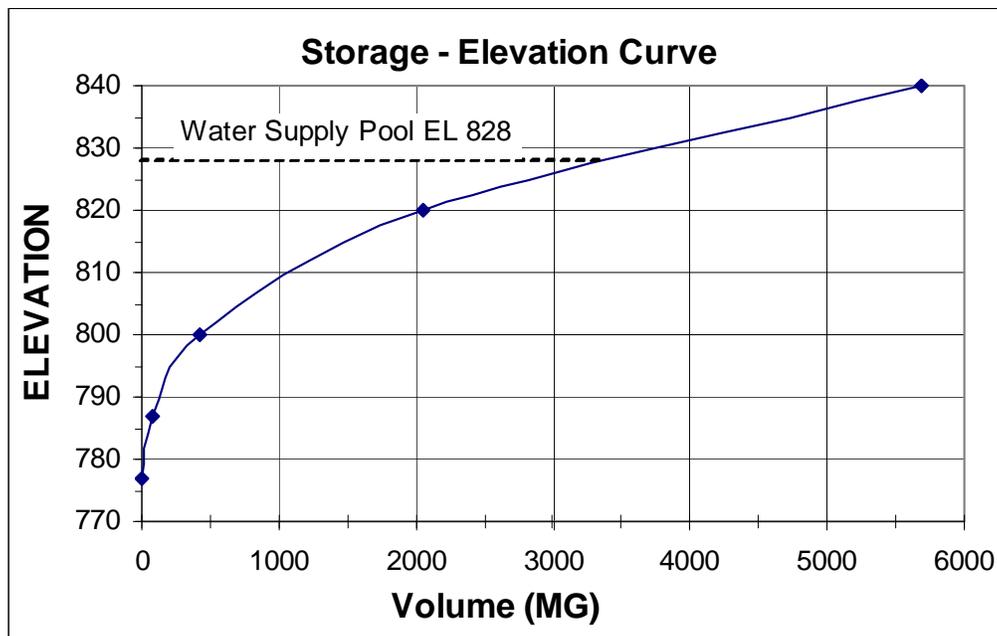
Figure 4
Watershed Location Map



Table 4
Summary of Reservoir Data

Stage	Elevation	Volume (Million Gallons)
Maximum Pool (Top of Dam)	840	5,700
Flood Pool (Auxiliary Spillway Crest)	830	3,750
Water Supply Pool	828	3,380

Figure 5



A reservoir operations model was developed to incorporate daily gage data from the selected USGS gage and reservoir shape parameters for estimation of evaporation. The following assumptions were incorporated into the analysis for the estimation of safe yield:

Assumptions:

1. Dead storage of 20% of gross reservoir storage was incorporated to allow for sediment storage and poor water quality in lower reservoir strata.
2. Usable water supply storage was assumed to be the water supply pool storage (calculated as noted above) less dead storage.
3. Pump station diversions were assumed to be from the Middle Oconee River at the location previously described. Diversions were assumed to occur whenever the reservoir level fell below full water supply pool. Pumped diversions were assumed to be bounded by pumping capacity and by flow restrictions on Middle

Oconee River (noted below).

4. A minimum in-stream flow (MIF) of 30% AAF at the diversion pump station (Middle Oconee River) was used.
5. Allowance for two downstream withdrawals on the Middle Oconee River (Upper Oconee Basin Water Authority (UOBWA) and the City of Athens) would increase release requirements at the proposed dam and diversion sites. In addition to the MIF, the model provided for a prorated let-by at the dam and proposed pump station (PS) with the following characteristics:

Permitee:	<u>UOBWA</u>	<u>Athens</u>
Downstream Withdrawal:	60 mgd	16 mgd
Drainage Area:	345 mi ²	390 mi ²
Prorated Let-by (Dam):	1.02 mgd	0.24 mgd
Prorated Let-by (PS):	57.9 mgd	13.7 mg

Additionally, Fall Line Hydro has a license for hydroelectric production downstream of the proposed diversion. According to GA EPD, protection of that flow is not required if it can be demonstrated that the proposed withdrawal will have negligible impact on the river flows. Such an analysis is beyond the scope of this report.

6. An upstream withdrawal by the City of Winder in the Middle Oconee River basin would reduce available flow in the diversion stream. Adjustments for Winder's withdrawal having the following characteristics were incorporated into the model:

Withdrawal:	5.1 mgd
Drainage Area:	109.7 mi ²
MIF:	9.7 mgd

7. For the dam site, minimum in-stream flow of 30/60/40 percent average annual flow (AAF) was used. This MIF applies as follows: 30% AAF for July through November; 60% AAF for January through April; and 40% AAF for May, June and December. The City of Statham has an existing 0.8 mgd permitted withdrawal from Barber Creek No. 6 Reservoir. For withdrawals up to this 0.8 mgd, there is no MIF requirement.
8. Return flow from wastewater discharges or septic systems was not considered in the analysis.
9. Evaporation loss was based upon net historical evaporation rates (one standard deviation above average monthly values) as recorded at the University of Georgia in Athens. Lake evaporation was assumed to be equal to 70% of pan evaporation during each month. Surface area was approximated by a regression equation relating storage to surface area (Figure A-2, Appendix).
10. Streamflow data from the USGS gages noted above was applied in direct proportion of drainage areas to simulate flow into the reservoir and at the diversion location.
11. Total seepage losses would be less than the MIF requirements and, therefore, did not need to be separately considered.
12. Safe yield is that quantity of water that can be provided to meet water demands during the critical drought period.

The attainable safe yield during the analyzed period was found by iteration of the daily mass balance equation:

$$\text{Ending Storage} = (\text{Beginning Storage}) + (\text{Natural Inflow}) + (\text{Pumped Inflow}) - (\text{Water Supply}) - (\text{Evaporation}) - (\text{MIF})$$

The trial safe yield value was varied until the reservoir level just reached the dead storage value, and recovery of the reservoir was computed.

SAFE YIELD RESULTS

Incorporating the above assumptions, the estimated safe yield of the site was computed. The results of the safe yield analysis are presented in Table 5 and Figure 6. It should be noted that these estimated safe yield values are based on USGS topographic mapping. The estimates could vary significantly based on more detailed mapping, which would be required as part of a final safe yield analysis. In addition, the final safe yield analysis should incorporate the most recent stream flow data to more accurately reflect the effects of the current drought (drought of record) on project safe yield. The table below presents the estimated safe yield and refill time for a range of pump capacities. We have assumed a refill time of 4 to 5 years is the maximum refill duration for selection of pump capacity.

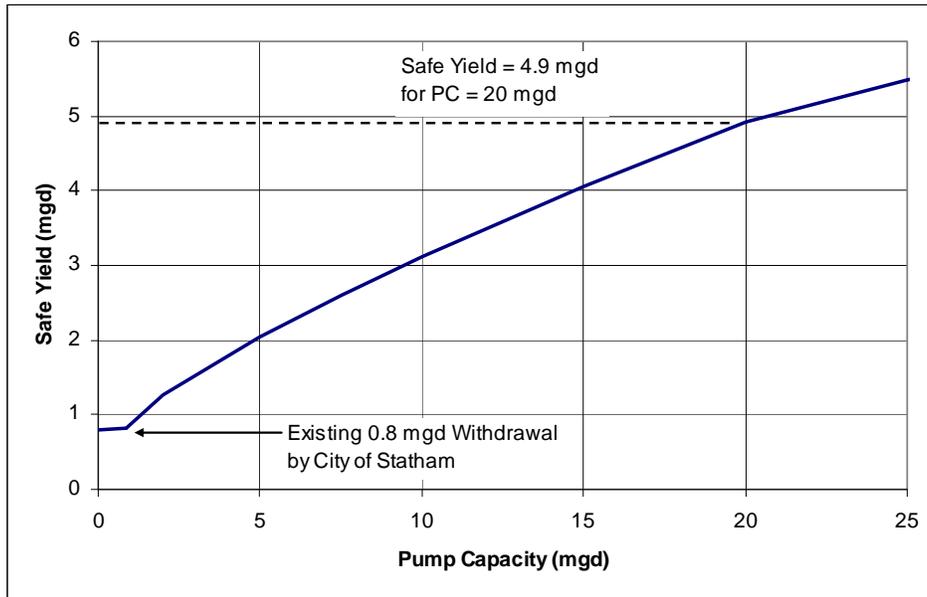
**Table 5
Safe Yield Summary**

Pump Capacity (mgd)	Estimated Safe Yield (mgd)	Refill Time** (years)
0	0.8*	4
2	1.3	14
5	2.0	8
10	3.1	6
15	4.0	5
20	4.9	4
25	5.5	4

*No MIF requirement for existing withdrawal up to 0.8 mgd.

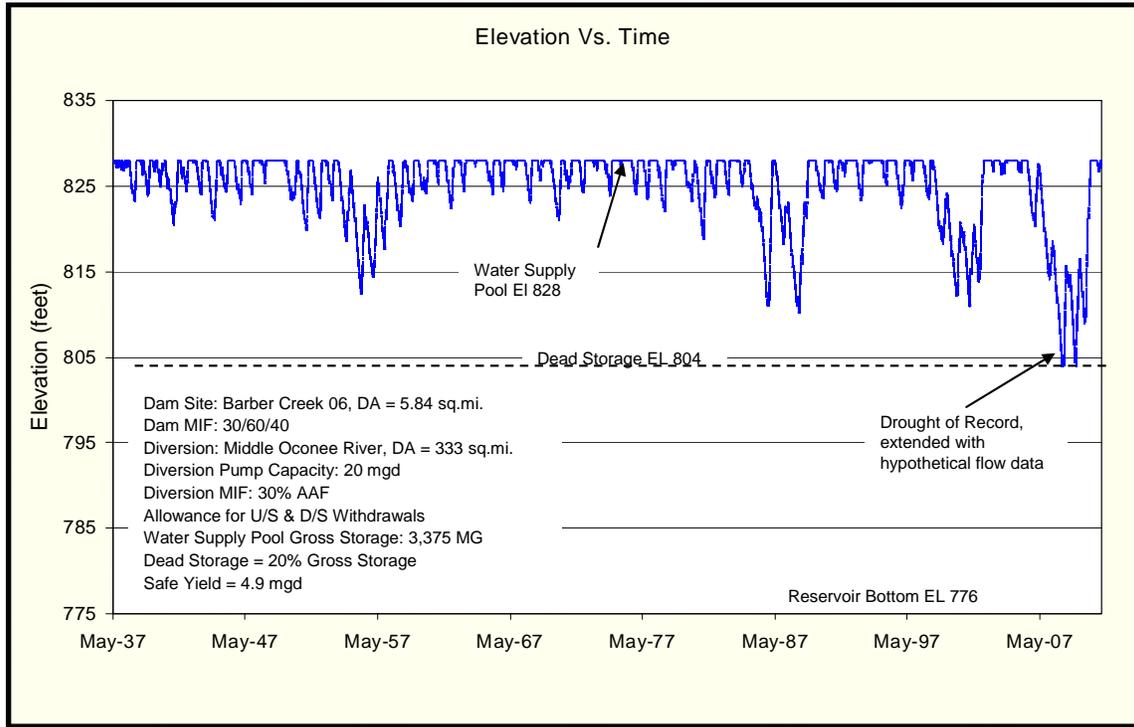
**Refill time is the time from start of drawdown until complete refill to water supply pool.

Figure 6
Estimated Safe Yield vs Pump Capacity



As presented in Figure 6, there is diminishing return (safe yield) with increasing pump capacity (reflecting pump station and pipeline cost). For the purposes of this analysis, an estimated economical safe yield and pump capacity combination were selected from the above graph. The estimated safe yield for this project is approximately 4.9 mgd for a pump capacity of 20 mgd. A relatively high pump capacity is required for this project due to the high releases required for existing downstream withdrawals. The selected pump capacity was used to size and cost out the diversion facilities detailed later in this report. The variation of reservoir elevation over time for the above assumed safe yield and pump capacity is reflected in Figure 7.

Figure 7
Reservoir Elevation vs. Time



ENVIRONMENTAL CONSIDERATIONS

To evaluate the potential environmental impacts, permitting and compensatory mitigation associated with Barber Creek 6, preliminary ecological studies were conducted by JIG ecologists. These studies consisted of a desktop survey to estimate wetlands, streams, and open waters (i.e. jurisdictional waters) occurring within the project area. All estimates of jurisdictional waters, permitting requirements, and compensatory mitigation requirements/cost estimates presented herein are very general and preliminary in nature. Detailed field studies would be necessary to definitively determine the number of jurisdictional waters and permitting requirements.

Desktop evaluations were performed with available data resources including the U.S. Geological Survey 7.5-minute topographic maps and U.S. Fish and Wildlife Service (USFWS) National Wetland Inventory (NWI) maps. In addition, current aerial maps were used to identify jurisdictional resources not indicated in any USGS or NWI data. Observations were transcribed into an ArcView GIS database for analysis. Preliminary estimates of jurisdictional waters occurring within the Barber Creek 6 project area are provided below.

Wetlands

The *Classification of Wetlands and Deepwater Habitats of the United States* (Cowardin Classification System) defines the Palustrine System as all nontidal wetlands dominated by trees, shrubs, persistent emergent vegetation, emergent mosses or lichens, and all such wetlands that occur in tidal areas where salinity is less than 0.5 percent. It also includes wetlands lacking such vegetation, but with all of the following four characteristics: 1) area less than 20-acres; 2) the lack of active wave-formed or bedrock shoreline; 3) water depth in the deepest part of basin less than 6.6 feet at low water; and 4) salinity due to ocean-derived salts less than 0.5 percent.

The Lacustrine System includes wetlands and deepwater habitats with all of the following characteristics: 1) situated in a topographic depression or a dammed river channel; 2) lacking trees, shrubs, persistent emergent vegetation, emergent mosses or lichens with greater than 30-percent areal coverage; and 3) total area exceeds 20 acres. Wetlands and deepwater habitats less than 20-acres are also included in this system if an active wave-formed or bedrock shoreline feature makes up all or part of the boundary, or if the water depth in the deepest part of the basin exceeds 6.6 feet at low water.

Office reviews determined that approximately 35 acres of palustrine wetlands and approximately 45 acres of lacustrine/palustrine open waters exist within the Barber Creek 6 project area. These systems are primarily associated with Barber Creek and several unnamed tributaries to Barber Creek within the proposed reservoir pool limits. Cowardin classifications of the wetland systems range from palustrine forested to palustrine emergent with hydrologic regimes ranging from saturated to seasonally flooded.

Streams

The Cowardin Classification System defines lower perennial streams as low gradient streams with slow water velocities and substrates comprised mainly of sand and mud. Intermittent

streams are defined as streams flowing for only part of the year. When water is not flowing, it may remain in isolated pools or surface water may be absent. Ephemeral streams flow only in direct response to precipitation and do not receive groundwater contributions.

Office reviews indicate that approximately 20,627 linear feet of lower perennial streams and approximately 18,716 linear feet of intermittent streams are located within the maximum reservoir pool limits of Barber Creek 6. Ephemeral streams were not identified due to the preliminary nature of the studies. Refer to Figure 8 for locations of these jurisdictional features.

Cultural Resources

Review of existing cultural resources information indicated one identified cultural resource site, a Watershed Lake Dam within the maximum reservoir pool limits of Barber Creek 6. This cultural resource appears to be the existing Barber Creek 06 dam embankment. A Phase I Cultural Resources Survey (conducted to the standards of Section 106 of the National Historic Preservation Act) and coordination with Georgia Historic Preservation Division would be required to determine potential cultural resources impacts for any proposed reservoir project.

Threatened and Endangered Species

Review of existing threatened and endangered species information identified three federally and state protected species documented from Barrow County, Georgia. These species consist of one faunal species and two floral species. The Georgia Department of Natural Resources – Non-game Conservation Section does not list the occurrence of any federally and state protected species within the maximum reservoir pool limits of Barber Creek 6. Specialized aquatic surveys would be required to definitively determine the presence/absence of the Altamaha shiner within the project area. Refer to Table 6 for a summary of protected species located in Barrow County and potential habitat for these species within the maximum reservoir pool limits.

Figure 8
Jurisdictional Areas Location Map

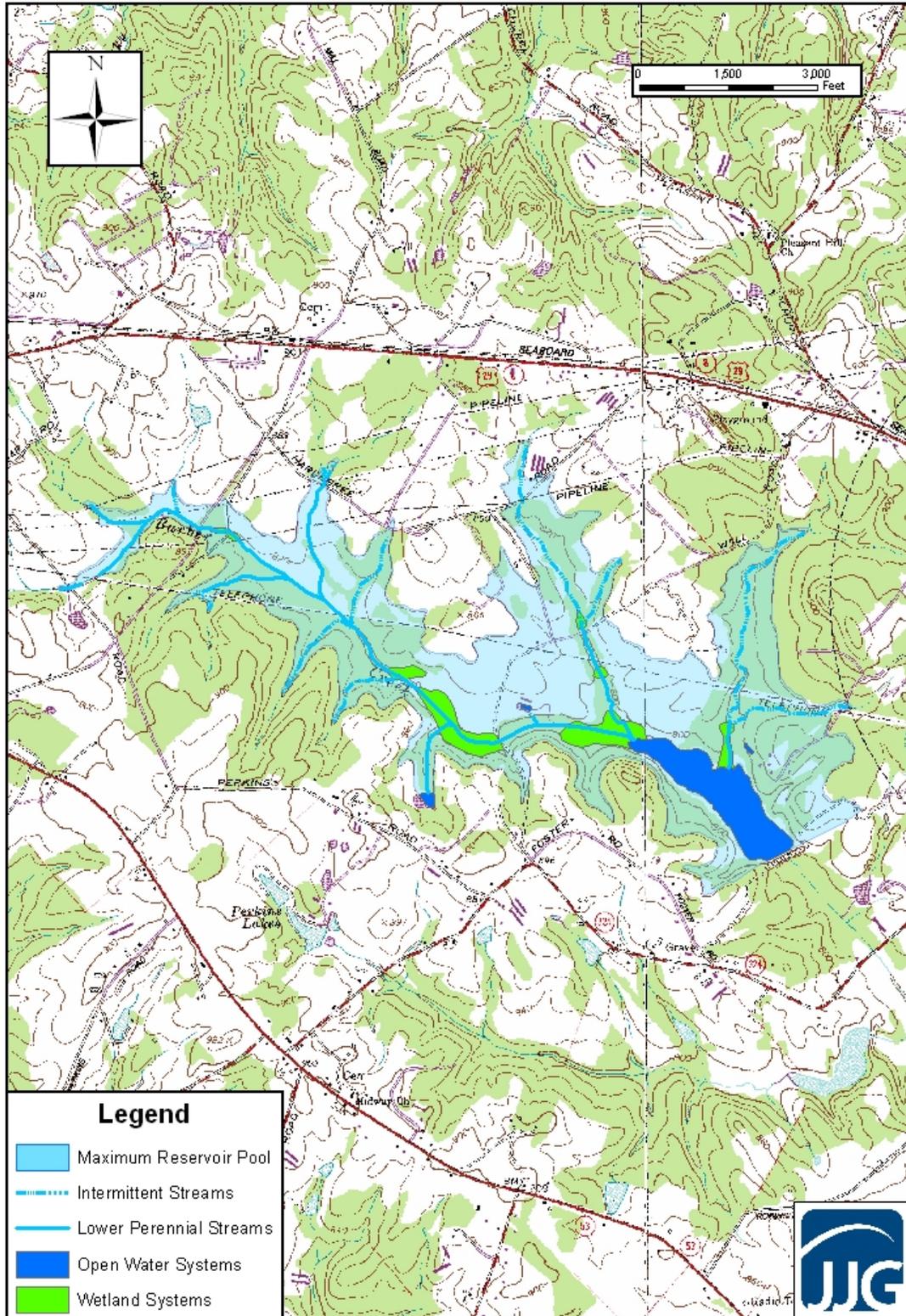


Table 6
Summary of Protected Species for Barrow County, Georgia

Scientific Name	Vernacular Name	Federal Status	State Status	Habitat Present (Yes/No)	Preferred Habitat
Faunal					
<i>Cyprinella xaenura</i>	Altamaha shiner	NA	T	Yes	small tributaries and rivers in pools with rocky to sandy substrates; Upper Altamaha river drainage
Floral					
<i>Cypripedium acaule</i>	pink ladyslipper	NA	U	Yes	upland oak-hickory-pine forests
<i>Sedum pusillum</i>	granite rock stonecrop	NA	T	No	granite outcrops among mosses in partial shade under eastern red cedars

T= threatened, U= unusual, NA= not applicable

Trout Streams

Review of available resources indicated no primary or secondary trout streams are located within the maximum reservoir pool limits of Barber Creek 6.

303(d) and 305(b) Listed Streams

Review of available resources did not indicate any 303(d) or 305(b) listed streams within the maximum reservoir pool limits of Barber Creek 6. However, it should be noted that the reach of Barber Creek immediately below the existing dam is 303(d) listed as “Not Supporting” due to fecal coliform.

Section 404/401 Permitting

The U.S. Army Corps of Engineers (USACE) regulates the discharge of dredged or fill material into the Nation’s Waters under Section 404 of the Clean Water Act. Construction of an impoundment and flooding jurisdictional streams and wetlands is regulated by the USACE. Two types of permits are available through the USACE: Nationwide and Individual Permits. Nationwide Permits (NWP) have been established previously by the Chief of Engineers for projects that have minimal cumulative impacts to the Nation’s Waters. Examples of the most

commonly used NWPs include site development, minor road crossings, maintenance activities, and utility line discharges. Specific criteria and conditions were established that must be satisfied prior to obtaining authorization of a NWP from the USACE. In addition, the USACE, Savannah District issued Regional Conditions effective May 11, 2007.

Individual Permits (IP) are required for projects having more than minimal cumulative adverse impacts on the Nation's waters. The development of a water supply reservoir would typically require an IP. IPs involve significantly more information, documentation, and coordination with regulatory agencies and are considerably more difficult to acquire than a NWP. Prior to coordination with the USACE regarding the construction of an impoundment, required information would consist of, but not be limited to, the following information:

- Justification of Purpose and Need for the project
- Alternatives analysis of other water supply options evaluated to meet the need
- Wetland delineation with surveyed boundaries of USACE jurisdictional waters
- Phase I cultural resources and protected species surveys
- Detailed description of proposed project and proposed impacts to jurisdictional waters
- Detailed analysis of flow releases documented with population analysis and system modeling
- Avoidance and minimization of jurisdictional waters analysis
- Identification of adjacent property owners
- Development of a conceptual compensatory mitigation plan

Following completion of these items, a complex project meeting would typically be scheduled with the USACE Northern Area Section Office (Morrow, GA) to present the proposed project. Subsequent to the meeting, and if a project is tentatively accepted by the regulatory agencies, preparation of an IP would begin along with the preparation of a formal application. Following submittal of an IP, the application must be advertised for public comment. The USACE prepares the public notice, which includes detailed applicant information such as site location, proposed impacts, cultural resources, protected species, and proposed mitigation. The public notice would be advertised for 30 days and is also submitted to regulatory agencies including the Environmental Protection Agency (EPA) and the USFWS, adjacent property owners, and to the USACE general mailing list. Applicants will be required to respond to inquiries received during the public notice process. Public hearings could be required if substantial adverse comments are received from the coordinating agencies or the public. Additional information and permitting required would consist of a Section 401 Water Quality Certification from the Georgia Environmental Protection Division (EPD). This certification must be issued for an IP to be valid. Depending on the level of impacts associated with the proposed reservoir, an Environmental Assessment or Environmental Impact Statement could be required by the USACE as well. Based on previous project experience, the level of controversy and environmental issues raised during agency and public review, a typical new reservoir project may require permitting times of five years or more.

The expansion of an existing reservoir could potentially facilitate the Section 404 permitting process when compared to the construction of a new impoundment. This is especially true for issues such as alternatives analysis, avoidance and minimization, and aquatic organism passage

in that many or most potential impacts have already occurred. However, the steps of the overall Section 404 permitting process would still need to be followed, and historically reservoirs have encountered significant regulatory and public challenges, regardless of the presence/absence of an existing impoundment.

Compensatory Mitigation

To determine the amount mitigation potentially required for jurisdictional impacts within the Barber Creek 6, the USACE's Standard Operating Procedure (SOP) for Compensatory Mitigation (March 2004) was utilized. The SOP uses a series of factors such as location, type, existing condition, type of impact, etc. to generate a multiplying "factor." That factor is then multiplied by the impact area (acreage or linear footage) to calculate the required mitigation credits. An "average" factor for jurisdictional areas associated with Barber Creek 6 was utilized. *However, it is imperative to note that this document only serves as a guideline if impacts do not exceed 5,000 linear feet of stream or ten acres of wetland impacts.* Potential impacts for the Barber Creek 6 would significantly exceed this threshold and actual compensatory mitigation requirements would likely be substantially different from SOP estimates. Currently, the USACE Savannah District Office is developing a new SOP for large-scale projects focused on reservoirs.

Utilizing the 2004 SOP and the approximated acreage and linear feet of jurisdictional waters located within the Barber Creek 6 project area, an estimate of compensatory mitigation credits can be determined. Multiplying factors used for this analysis include: 6.7 for wetland systems, 5.7 for open waters, 12.7 for lower perennial streams, and 7.6 for intermittent streams. This factor was then multiplied by the acreage/ linear footage to determine an estimated number of mitigation credits required. The number of credits was then multiplied by an average credit price to estimate the final estimated compensatory mitigation cost associated with the Barber Creek 6. Refer to Table 7 in the next section for estimated impacts to jurisdictional waters and an estimate of mitigation credits required and associated costs.

Stream Buffer Variance

The Georgia Erosion and Sedimentation Act of 1975 (GESA), as amended, requires that a 25-foot vegetated buffer be maintained along all state waters. Any land disturbing activities within the buffer would require obtaining a stream buffer variance from the EPD. The local issuing authority is responsible for determining if state waters are on-site and is responsible for determining if a stream buffer variance is required.

The GESA has a number of activities that are considered for stream buffer variances, including public water system reservoirs. Based on current regulations, reservoir construction would likely qualify for a variance. Attendant features such as pipelines and roadways, would likely be exempt from GESA regulations if stream crossings are constructed nearly perpendicular.

EPD Water Withdrawal Permit

Georgia EPD requires a permit for withdrawal of 100,000 gallons per day or more of either surface water or ground water. In addition to justification of water needs for up to 50 years in the future, water withdrawal permits typically require the preparation of water conservation, drought contingency, water supply/watershed protection, and reservoir management plans. A public hearing may be required as part of the withdrawal permitting process. EPD requires that its comments on the component plans be addressed before moving forward with issuing the water withdrawal permit. Based on previous permitting experience, a water withdrawal permit can be obtained within 5 to 7 months, depending on EPD's review time and the extent of their comments.

Source Water Protection Plan

Amendments to the Federal Safe Drinking Water Act (SDWA) have brought about a new approach for ensuring clean and safe drinking water served by public water supplies in the United States. Management of a drinking water source now requires a Source Water Protection Plan. This plan basically defines watershed management strategies for ensuring that the water supply is not compromised by potential pollutant sources. Typically these sources are unmanaged development, but they can also include industrial sources that can potentially contaminate the water supply. The entity that operates this reservoir for water supply would be required to produce and implement the Plan. The Plan should also address any source water from outside the reservoir watershed that would be used to fill the reservoir, i.e., pumped/storage sources. The cost and schedule for producing a Source Water Assessment and the corresponding Source Water Protection Plan have not been included in the estimates presented in the report.

PROJECT CONSTRUCTION COST ESTIMATE NARRATIVE

Dam and Reservoir

The construction cost estimate for the proposed dam was based upon the general description provided in the background section of the report. Additionally, the following assumptions were made regarding the geometry of the dam.

- Upstream slope of 3H to 1V
- Downstream slope of 3H to 1V
- Upstream slope wave action protection in the form of riprap from 30 feet below the crest of the dam to 5 feet below the crest of the dam. Riprap supported by a berm located 30 feet below top of dam.
- Downstream slope having nearly horizontal 12-foot wide berms at 30-foot vertical intervals to control surface water runoff and erosion
- Crest of dam having a width of 25-feet

In addition to the above geometric considerations, the following internal drainage configurations were also considered in the estimation of construction costs.

- Chimney drain located at the downstream edge of the crest
- Trench drain located at 1/3 the distance from the downstream toe to the crest

A plan view and cross section of the proposed dam is provided in Figures 9 and 10.

Contained below are the items estimated to develop the construction cost estimate. We caution that the quantities and associated prices are based upon limited engineering evaluation and will likely change as the project proceeds into detailed evaluation and design.

Mobilization and Demobilization

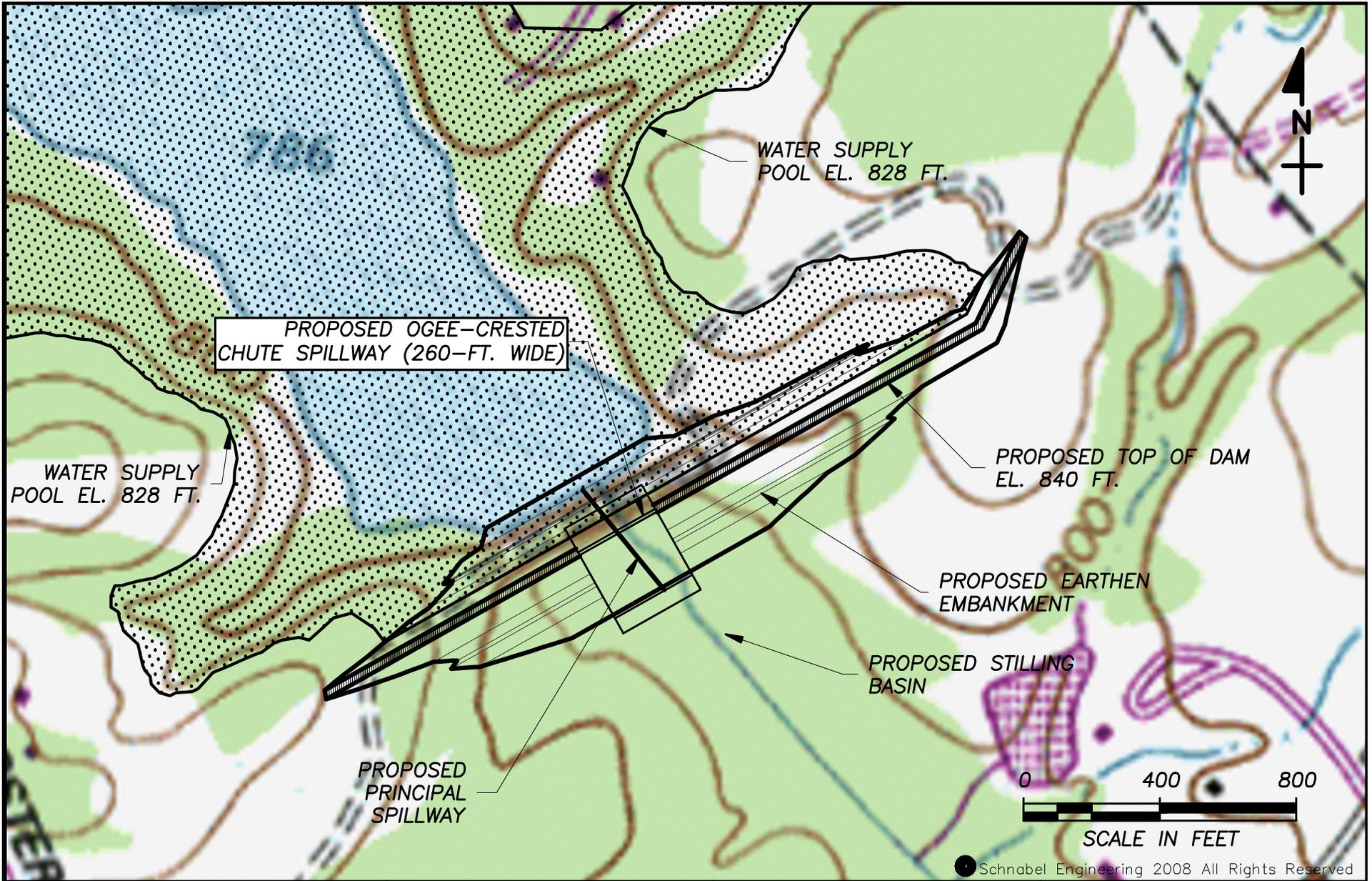
Mobilization and demobilization is a lump sum item estimated at 6 percent of the unit rate sum of the construction items.

Erosion and Sedimentation Control

Erosion and sedimentation control is a lump sum item estimated at 2 percent of the sum of unit rate construction items.

Control of Water

Control of water is a lump sum item estimated at 3 percent of the sum of unit rate construction items. This item includes the control of both surface water and groundwater and will likely consist of stream diversion, cofferdam construction and maintenance, pumping, and well points, as well as any other means of controlling water during construction.



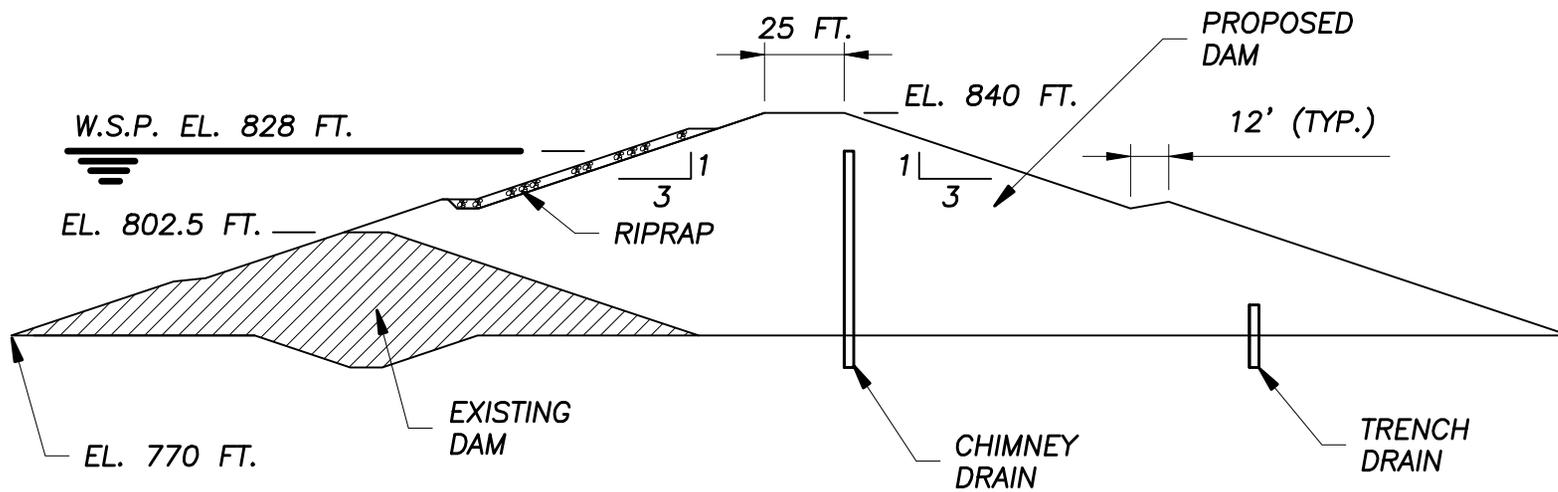
G:\2007 Jobs\07170030\06 Assessment\Barber Creek 06\CAD\BARBER CREEK 06 - FIGURE 9.dwg



NJCS WATERSHED DAM ASSESSMENT
BARBER CREEK DAM NO. 06

PROPOSED PLAN VIEW
PROJECT NO. 07170030.06
FIGURE 9

GA:\2007 Jobs\07170030\06 Assessment\Barber Creek 06\CAD\BARBER CREEK 06 - FIGURE 10.dwg



SCALE IN FEET

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NCS WATERSHED DAM ASSESSMENT
BARBER CREEK DAM NO. 06

TYPICAL SECTION

PROJECT NO. 07170030.06
FIGURE 10

Clearing

Clearing is a unit rate item measured in acres associated with the removal of trees and other vegetation from the reservoir. The estimated area of clearing was assumed to be equal to the surface area of the reservoir at the normal pool elevation.

Clearing and Grubbing

Clearing and grubbing is a unit rate item measured in acres associated with the removal of trees, other vegetation, and associated root mats in the areas to receive structural fill or concrete. The estimated area of clearing and grubbing was assumed to be equal to the footprint of the proposed dam plus an additional 50-foot perimeter around the proposed dam.

Earth Fill

Earth Fill is a unit rate item measured in cubic yards. The computed volume of earth fill represents the estimated quantity required to construct the dam as described herein. The estimated quantity was computed using an AutoCad Civil 3D computer model based on the proposed grading and existing topography. In addition to the proposed embankment earth fill, foundation excavation backfill was calculated (see Excavation, Common for details) and added to the embankment earth fill to determine the total quantity of earth fill.

Drain Fill

Drain Fill is a unit rate item measured in cubic yards. The computed volume of drain fill represents the estimated quantity of fine and coarse-grained drain material required to construct the internal drainage system as described herein. For the purposes of this study, no differentiation was made between fine and coarse drain fill. In addition, the quantity for the trench drain was assumed to be equal to half of the chimney drain quantity. The chimney drain was assumed to have a top elevation equal to the proposed normal pool elevation and a bottom elevation approximated at the limits of the foundation excavation. The chimney drain was assumed to have a width of three feet and run the length of the dam from one abutment, into the floodplain, and up the other abutment tying into residual soils.

Excavation, Common

Excavation, Common is a unit rate item measured in cubic yards associated with the removal of unsuitable material (soils) within and adjacent to the footprint of the proposed dam. The volume of common excavation was calculated by approximating the surface area of the floodplain within the limits of clearing and grubbing as well as the depth of excavation within the same area. The surface area of the floodplain was approximated using available topographic maps. The depth of excavation was estimated from the boring data included in the design plans for the existing dam.

Riprap

Riprap is a unit rate item measured in tons. The computed weight of riprap represents the estimated quantity required to construct the wave-action berm as described herein. Riprap was assumed to be placed on the upstream slope of the dam. The section of riprap was assumed to extend 30 vertical feet, have a thickness of about 2-¾ feet, and traverse the length of the proposed dam.

Permanent Turf Establishment

Permanent Turf Establishment is a unit rate item measured in acres associated with the establishment of a permanent turf at the conclusion of construction activities for the proposed dam. The estimated area of permanent turf establishment was assumed to be equal to the estimated area of clearing and grubbing.

Concrete, Class 4000

Concrete, Class 4000 is a unit rate item measured in cubic yards associated with the construction of the reinforced concrete auxiliary chute spillway. The volume of concrete was estimated by comparing the proposed auxiliary spillway drop in elevation and width to the drops in elevation and widths of constructed reinforced concrete chute spillways. A relationship was developed between the drop in elevation and width of the constructed spillways and the required quantity of concrete. This relationship was applied to the proposed dam to estimate the quantity of concrete.

Principal Spillway Reinforced Concrete Pressure Pipe

Reinforced Concrete Pressure Pipe (RCPP) is a unit rate item measured in feet. The computed length of RCPP represents the estimated quantity required to construct the principal spillway conduit described herein. The RCPP was assumed to be placed through the base of the proposed dam from the upstream toe to the downstream toe. The diameter of the pipe was assumed to be equal to the diameter of the pipe in the existing dam.

Concrete, Class 3000 (mass)

Concrete, Class 3000 is a unit rate item measured in cubic yards associated with the construction of the concrete cradle beneath the principal spillway pipe. The concrete cradle was assumed to be designed as a Soil Conservation Service Type A2 cradle and run the length of the principal spillway pipe minus ten feet.

Reinforced Concrete Riser

The Reinforced Concrete Riser is a lump sum item associated with the construction of the reinforced concrete principal spillway structure. The cost was estimated by comparing the proposed principal spillway riser height to the heights of constructed reinforced concrete riser structures. A relationship was developed between the height of the constructed spillways and the

cost to construct them. This relationship was utilized to estimate the cost of the proposed riser structure.

Land Acquisition

The costs associated with land acquisitions are unit rate items based upon the number of acres that will need to be purchased at the top-of-dam elevation, the number of acres that will need to be managed for a 150-foot buffer around the normal pool, and the number of houses that will need to be purchased. For the purposes of the buffer management, only the portions of the buffer above top-of-dam elevation were considered. The costs to purchase the land were estimated based upon available records of recent land sales. The cost to manage the buffer was assumed to be 60 percent of the land purchase cost. The cost of each structure impacted was assumed to be \$200,000.

Roadway Relocation

To construct the proposed project, eight roads will be impacted. These roads may need to be raised, relocated, or modified to accommodate the new reservoir; however, no consideration was given to the relocation of the roads in this study. A more detailed evaluation would need to be performed to evaluate the impact on existing roadways and the associated cost.

Utility Relocation

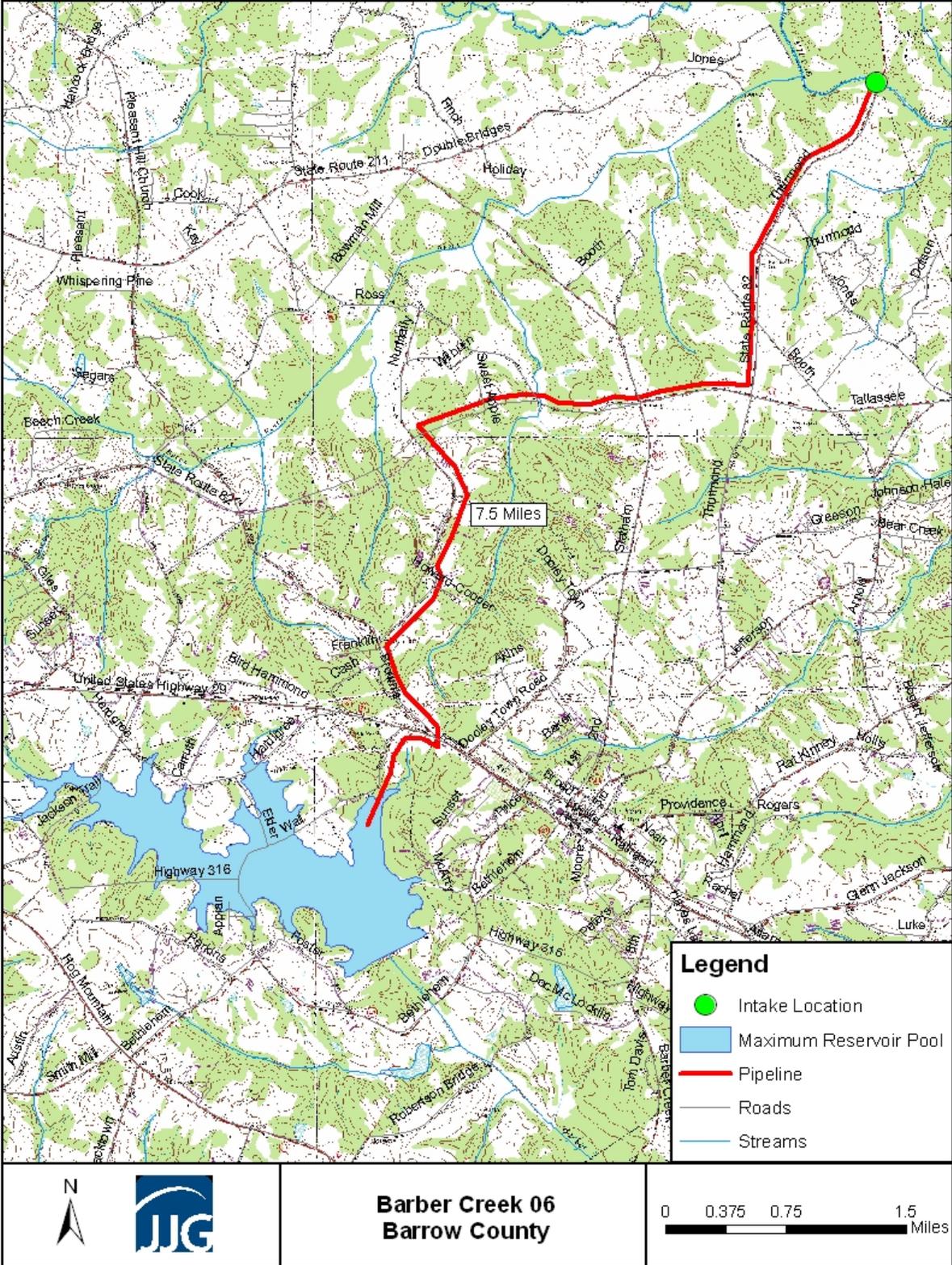
To construct the proposed project a natural gas pipeline and an telephone transmission line will be impacted; however, no consideration was given to the impact these will have. A more detailed evaluation would need to be performed to evaluate the impact and cost associated with these utilities.

Pump Station and Pipeline Cost Estimation

The pump storage location for Barber Creek Reservoir 06 is located on the Middle Oconee River at the State Route 82 crossing as shown in Figure 11. The reservoir is located approximately 6 miles southwest on Barber Creek. With a normal pool elevation of 828 feet, Reservoir 06 has an average day yield of approximately 4.9 MGD. A 36-inch pipeline was selected to carry water from the pump storage location to the reservoir. This pipeline is approximately 7.5 miles in length and will pump water from the storage location elevation of 660 feet, to the 828 feet height of the reservoir water surface. A cascading structure will need to be constructed where the pipe comes into the reservoir to provide aeration and erosion control.

Three 10-MGD pumps were selected at the pump storage location to pump water to the reservoir, giving a firm pumping capacity of 20-MGD. An access road will need to be constructed in order to construct and maintain the pumping station on the Middle Oconee River. This road will only need to run approximately 250 feet from State Route 82. The cost opinion for these components is found in the appendix.

**Figure 11
Project Location Map**



Compensatory Mitigation

The simplest mitigation option is typically purchasing credits from a bank. Compensatory mitigation credits may be purchased from an approved mitigation bank or through the Georgia Land Trust Service Center if a bank is not available within the project area. Based on recent projects, wetland credits range from \$7,000-\$10,000 per credit and stream credits range from \$70-\$110 per credit. An option to purchasing credits is to obtain credits by conducting on-site restoration or preservation of jurisdictional waters.

**Table 7
Barber Creek 6 Estimated Impacts and Overall Mitigation Banking Cost Analysis**

Impact Type	Estimated Impact Acres/Linear Feet	Projected Credits Needed	Projected Cost* \$90/stream credit \$7,500/wetland credit
Wetland	34.62 A	232	\$1,740,000
Intermittent Stream	18,716 l.f.	142,242	\$12,801,780
Lower Perennial Stream	20,627 l.f.	261,963	\$23,576,670
Open Water	44.73 A	255	\$1,912,500
Total	79.35 acres / 39,343 lf	487 wetland / 404,205 stream**	\$40,030,950

*Cost is based on recent quotes from banks within the Upper Oconee River Basin. Actual banking price may be higher or lower than estimated depending on the date of purchase and credit availability. **Total required credits calculated using the March 2004 Standard Operating Procedure mitigation guidelines established by the US Army Corps of Engineers.

Estimated Project Construction Cost

The total project cost is estimated at \$90,000,000. Table A-5, located in the appendix, shows an itemized breakdown of the costs associated with enlarging the existing dam and reservoir. These costs are estimates and are based on multiple assumptions.

APPENDIX

FIGURES

Figure A-1	Stage Storage / Stage Area Curves
Figure A-2	Regression Equations for Area to Storage and Depth to Storage
Figure A-3	Storage vs. Time and Elevation vs. Time for Assumed Safe Yield

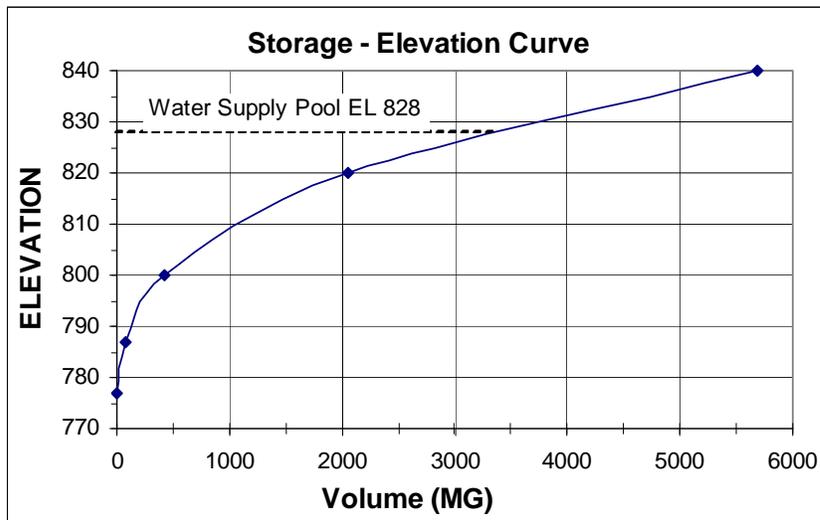
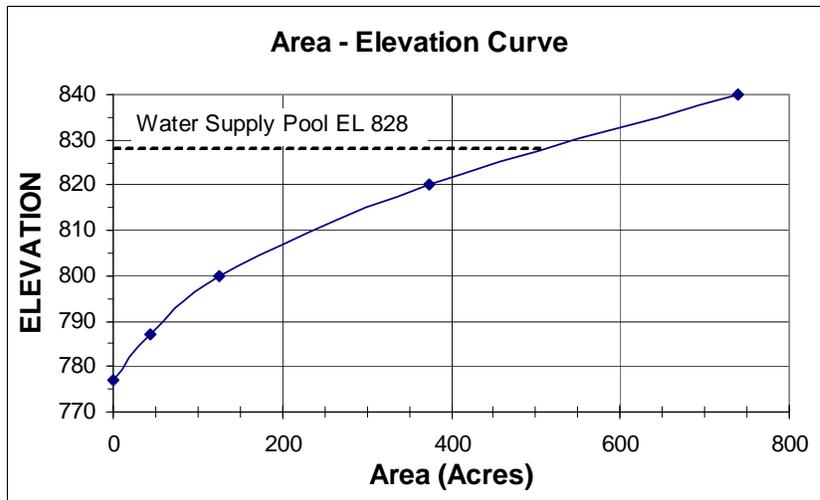
TABLES

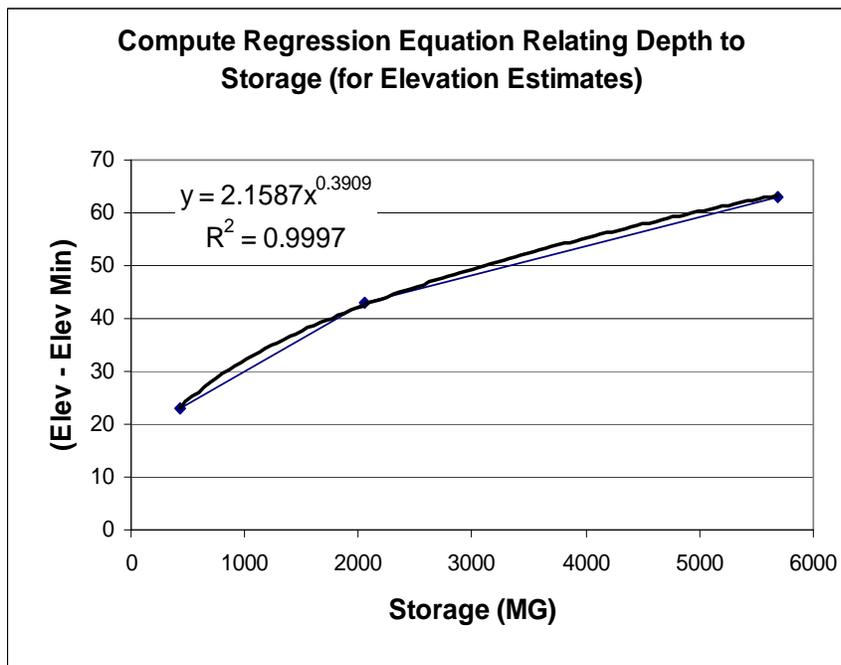
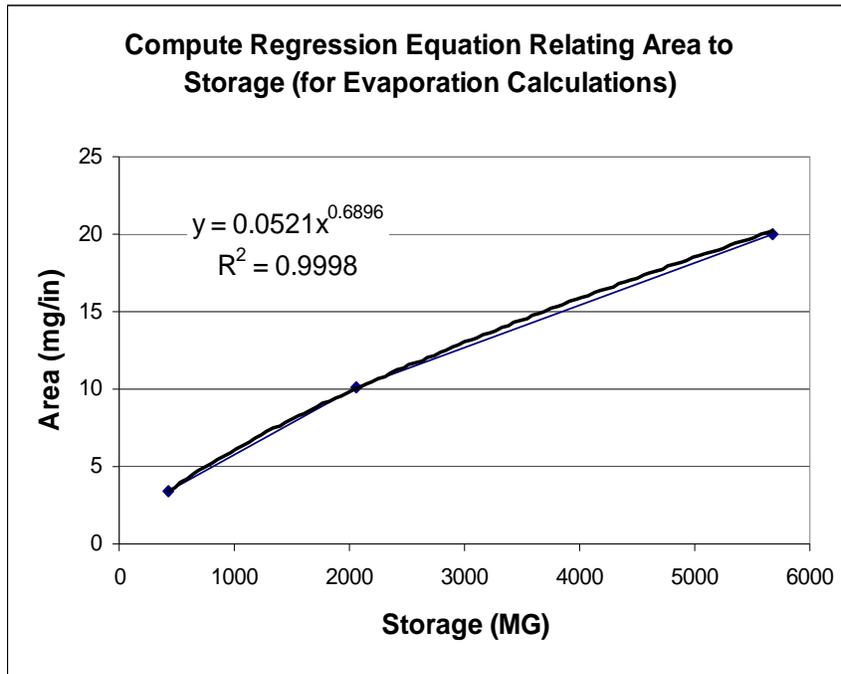
Table A-1	Summary of Opinion of Probable Construction Costs for Pumping Facilities and Pipelines
Table A-2	Opinion of Probable Construction Costs – River Intake and Pump Station
Table A-3	Opinion of Probable Construction Costs – 30-inch Raw Water Line
Table A-4	Opinion of Probable Construction Costs – Reservoir Inlet Structure
Table A-5	Total Project Opinion of Cost

Figure A-1

Barber Creek 06 Area and Storage Curves

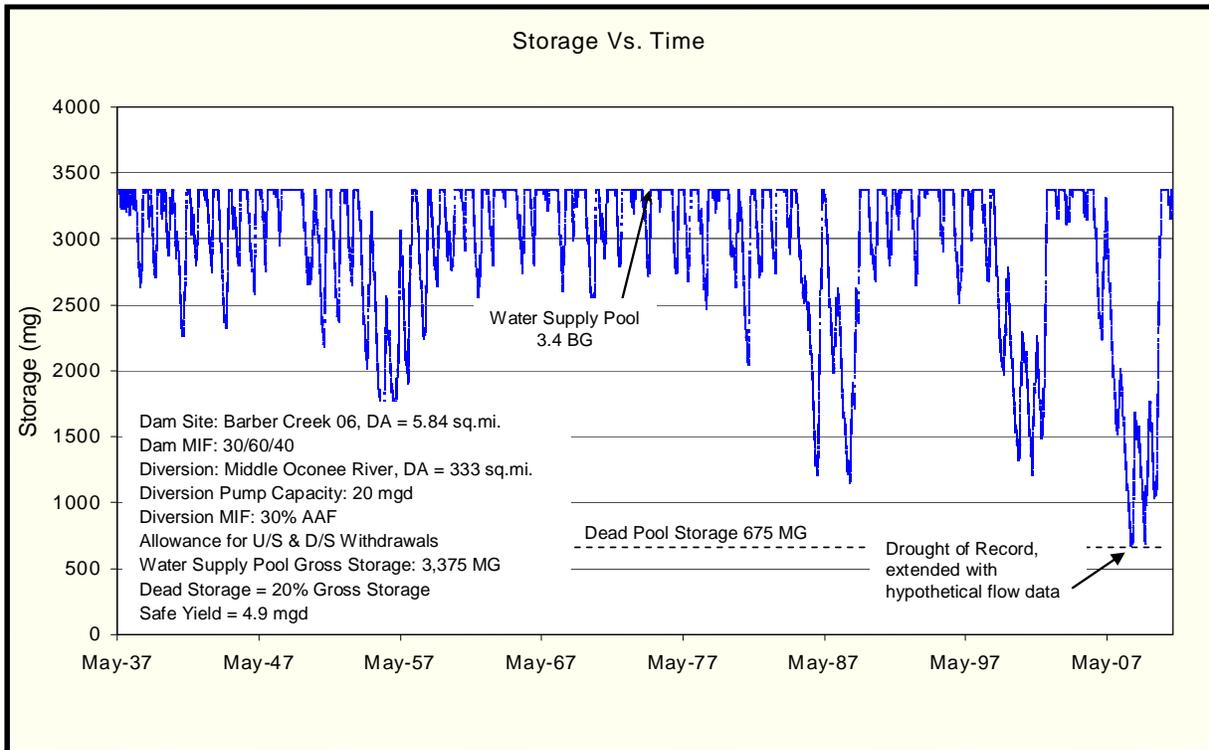
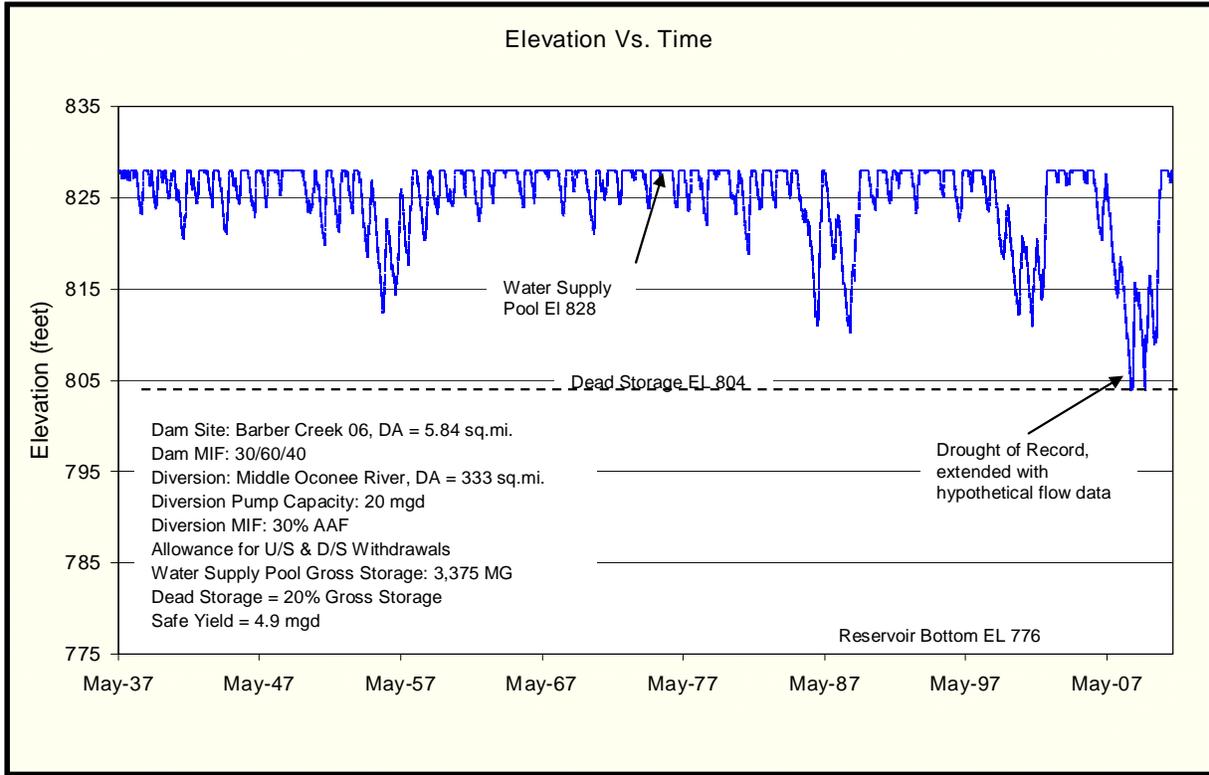
Elev.	Area Acres	Area mg/in	Inc. Vol. A-FT	Cumulative Vol A-FT	M Gal.
777	0.0	0	0	0	0
787	44	1	220	220	72
800	124.8	3	1097	1317	429
820	374.0	10	4987	6304	2055
840	738.8	20	11128	17432	5681





Barber Creek 06

Figure A-3



WATERSHED DAM ASSESSMENT - BARBER CREEK 06
Barrow County, Georgia (7194-002)
OPINION OF PROBABLE CONSTRUCTION COST ESTIMATE - CONCEPTUAL LEVEL
Summary by Division

Table A-1

Division	01 - Intake and Pump Station and Access Road	02 - 36 - inch Raw Water Force Main and Venturi Vault	03 - Reservoir Inlet Structure	TOTAL	% of Total	
1	\$0.74	\$0.92	\$0.08	\$1.74	7.93%	BARBER CREEK 06
2	\$0.60	\$1.20	\$0.04	\$1.85	8.42%	
3	\$0.75	\$0.02	\$0.39	\$1.15	5.23%	
4	\$0.11	\$0.00	\$0.00	\$0.11	0.49%	
5	\$0.02	\$0.00	\$0.00	\$0.02	0.10%	
6	\$0.00	\$0.00	\$0.00	\$0.00	0.00%	
7	\$0.02	\$0.00	\$0.00	\$0.02	0.10%	
8	\$0.03	\$0.00	\$0.00	\$0.03	0.14%	
9	\$0.05	\$0.00	\$0.00	\$0.05	0.23%	
10	\$0.00	\$0.00	\$0.00	\$0.00	0.00%	
11	\$2.33	\$0.00	\$0.03	\$2.35	10.73%	
12	\$0.00	\$0.00	\$0.00	\$0.00	0.00%	
13	\$0.00	\$0.00	\$0.00	\$0.00	0.00%	
14	\$0.12	\$0.00	\$0.00	\$0.12	0.53%	
15	\$0.51	\$8.06	\$0.02	\$8.59	39.17%	
16	\$0.77	\$0.07	\$0.06	\$0.90	4.09%	
17	\$0.24	\$0.05	\$0.03	\$0.31	1.41%	
Structure Contingency	\$0.94	\$0.51	\$0.03	\$1.49	6.79%	
Markup	\$1.25	\$1.82	\$0.14	\$3.21	14.63%	
Structure Total (without Contingency)	\$8.48	\$12.64	\$0.81	\$21.93	100.00%	
Project Contingency	\$2.54	\$3.79	\$0.24	\$6.58	30.00%	
Structure Total (with Contingency)	\$11.02	\$16.43	\$1.06			
All Figures are in Millions	PROJECT TOTAL			\$28.51 M		

WATERSHED DAM ASSESSMENT - (7194-002)

BARBER CREEK 06

OPINION OF PROBABLE CONSTRUCTION COST ESTIMATE - CONCEPTUAL LEVEL

01
DECEMBER 2008

Table A-2

No.	Spec. Sect.	Description	Unit	Qty	Labor \$\$		Material \$\$		Equipment \$\$		Subcontractor \$\$		Total
					Unit	Total	Unit	Total	Unit	Total	Unit	Total	
01 - Barber Creek 06: River Intake and Pump Station					3 - Channel Intake Pump Station					Pump Station Firm Capacity is 20.0 MGD			
Div 1													
1	1000	General Conditions	LS	1		\$266,000		\$210,600		\$266,000		\$0	\$742,600
Div 2													
2	2200	Earth Work	LS	1	\$18,000.00	\$18,000	\$10,900.00	\$10,900	\$13,055.00	\$13,060	\$298,300.00	\$298,300	\$340,260
3		Access Road	LF	250		\$0		\$0		\$0	\$110.00	\$27,500	\$27,500
4	2831	10' Galv. Chain Link Fence	LF	1200		\$0		\$0		\$0	\$30.00	\$36,000	\$36,000
5	2831	Dewatering / Pre-Excavation Preparation	LS	1	\$50,000.00	\$50,000	\$20,000.00	\$20,000	\$100,000.00	\$100,000	\$30,000.00	\$30,000	\$200,000
Div 3													
6	3250	Water Stop	LF	500	\$1.25	\$630	\$2.00	\$1,000		\$0		\$0	\$1,630
7	3300	Concrete Bridge	SF		\$2.00	\$0		\$0	\$3.50	\$0	\$20.00	\$0	\$0
8	3300	Concrete	LS	1	\$228,927.00	\$228,930	\$445,530.00	\$445,530	\$71,000.00	\$71,000	\$0.00	\$0	\$745,460
Div 4													
9	4210	Brick Veneer	SF	3760		\$0		\$0		\$0	\$17.50	\$65,800	\$65,800
10	4220	Concrete Masonry Unit - Reinforced	SF	3760		\$0		\$0		\$0	\$11.00	\$41,360	\$41,360
Div 5													
9	5524	Aluminum Handrail	LF	200	\$6.00	\$1,200	\$35.00	\$7,000	\$2.90	\$580		\$0	\$8,780
10		Ladder	VF	20	\$50.00	\$1,000	\$150.00	\$3,000	\$15.00	\$300		\$0	\$4,300
11	5530	Aluminum Grating Landing	SF	64	\$10.00	\$640	\$45.00	\$2,880	\$10.00	\$640		\$0	\$4,160
12	5530	Aluminum Grating	SF	160	\$10.00	\$1,600	\$20.00	\$3,200		\$0		\$0	\$4,800
Div 6													
Div 7													
13		Membrane Roofing	SF	1260		\$0		\$0		\$0	\$10.00	\$12,600	\$12,600
14		Dampproofing - Walls	SF	3760		\$0		\$0		\$0	\$0.56	\$2,110	\$2,110
15		1" Rigid Insulation - Walls	SF	3760		\$0		\$0		\$0	\$1.07	\$4,020	\$4,020
16	7210	Walls - Core Fill Foam Insulation (12" CMU)	SF	3760		\$0		\$0		\$0	\$0.61	\$2,290	\$2,290
Div 8													
17	8120	Hollow Metal Doors, Hardware, and Frames - Single	EA	10	\$150.00	\$1,500	\$400.00	\$4,000		\$0		\$0	\$5,500
18	8120	Hollow Metal Doors, Hardware, and Frames - Double	EA	2	\$150.00	\$300	\$800.00	\$1,600		\$0		\$0	\$1,900
19		Windows	LS	1	\$3,000.00	\$3,000	\$8,000.00	\$8,000	\$1,000.00	\$1,000		\$0	\$12,000
20	8331	Roll Up Aluminum Door (10'x12')	EA	2	\$800.00	\$1,600	\$4,500.00	\$9,000	\$50.00	\$100		\$0	\$10,700
Div 9													
21	9900	Painting	LS	1		\$0		\$0		\$0	\$50,000.00	\$50,000	\$50,000
Div 10													
Div 11													
22		Screens / Spray Water System and Strainer	EA	3	\$20,000.00	\$60,000	\$322,500.00	\$967,500	\$2,500.00	\$7,500		\$0	\$1,035,000
23		Eductors	EA	18	\$200.00	\$3,600	\$3,500.00	\$63,000	\$50.00	\$900		\$0	\$67,500
24		Pumps (10.0 MGD, 360 Feet TDH)	EA	3	\$30,000.00	\$90,000	\$375,000.00	\$1,125,000	\$2,500.00	\$7,500		\$0	\$1,222,500
Div 12													
Div 13													
Div 14													
25		Bridge Crane	LS	1	\$5,000.00	\$5,000	\$110,000.00	\$110,000	\$1,500.00	\$1,500		\$0	\$116,500
Div 15													
26	15062	Ductile Iron Pipe	LS	1	\$13,116.00	\$13,120	\$268,944.79	\$268,940	\$3,150.00	\$3,150	\$0.00	\$0	\$285,210
27		PVC Piping	LS	1	\$1,250.00	\$1,250	\$8,000.00	\$8,000	\$750.00	\$750		\$0	\$10,000
28		Valves	LS	1	\$11,800.00	\$11,800	\$123,600.00	\$123,600	\$5,200.00	\$5,200	\$0.00	\$0	\$140,600
29		HVAC and Plumbing	LS	1		\$0		\$0		\$0	\$70,000.00	\$70,000	\$70,000
Div 16													
30	16000	Electrical	LS	1		\$0		\$0		\$0	\$720,000.00	\$720,000	\$720,000

WATERSHED DAM ASSESSMENT - (7194-002)

01

BARBER CREEK 06

DECEMBER 2008

OPINION OF PROBABLE CONSTRUCTION COST ESTIMATE - CONCEPTUAL LEVEL

No.	Spec. Sect.	Description	Unit	Qty	Labor \$\$		Material \$\$		Equipment \$\$		Subcontractor \$\$		Total
					Unit	Total	Unit	Total	Unit	Total	Unit	Total	
01 - Barber Creek 06: River Intake and Pump Station					3 - Channel Intake Pump Station				Pump Station Firm Capacity is 20.0 MGD				
31		CCTV Allowance	LS	0		\$0		\$0		\$0		\$0	\$0
32		Ductbank	LF	350		\$0		\$0		\$0	\$150.00	\$52,500	\$52,500
Div 17													
33	17000	Instrumentation	LS	1		\$0		\$0		\$0	\$240,000.00	\$240,000	\$240,000
		Contingency	LS	15%		\$114,000		\$509,000		\$72,000		\$248,000	\$943,000
Subtotals						\$873,170		\$3,901,750		\$551,180		\$1,900,480	\$7,226,580
					Assumptions:								
Sales Tax @					7.0%	\$273,100	Assumes that EPD will allow withdrawal from this source						
Labor Burden @					30.0%	\$262,000	15 foot wide Asphalt access road with 10-foot high fence						
Bonds On Subs @					1.5%	\$28,500	Pump Station firm capacity is 15MGD						
Subtotal						\$7,790,180	Pump Station has a 3 channel intake						
Fee @					7.0%	\$545,300	Pump Station footprint is approximately 100 feet by 40 feet						
Insurance & Bonds @					1.7%	\$141,700	Pump Station main building footprint is approximately 35 feet by 35 feet						
							Pump Station main building also houses the electrical room and is made of brick and block						
Estimated Construction Cost						\$8,480,000	A Transformer is being provided by the Utility Company at the access road entrance						
							Estimate DOES NOT include easements acquisitions, land acquisitions, withdrawal permits or mitigations required to build the pump station						

WATERSHED DAM ASSESSMENT - (7194-002)
BARBER CREEK 06
OPINION OF PROBABLE CONSTRUCTION COST - CONCEPTUAL

02
 DECEMBER 2008
 Table A-3

No.	Spec. Sect.	Description	Unit	Qty	Labor \$\$		Material \$\$		Equipment \$\$		Subcontractor \$\$		Total
					Unit	Total	Unit	Total	Unit	Total	Unit	Total	
02 - 36-inch Raw Water Line with Venturi Vault													
Div 1													
1	1000	General Conditions	LS	1		\$338,000		\$244,100		\$337,900		\$0	\$920,000
Div 2													
2	2125	Erosion and Sedimentation Control Maintenance	LS	1		\$0		\$0		\$0	\$560,100.00	\$560,100	\$560,100
3		Bore and Jack Road Crossing (48")	LF	550		\$0		\$0		\$0	\$600.00	\$330,000	\$330,000
4	2510	Asphalt Concrete Pavement (5% of length)	LS	1		\$0		\$0		\$0	\$206,300.00	\$206,300	\$206,300
5	2523	Driveway Replacement (62 total)	LS	1		\$0		\$0		\$0	\$103,500.00	\$103,500	\$103,500
Div 3													
6	3300	Miscellaneous Concrete (Venturi Vault)	LS	1	\$1,500.00	\$1,500	\$12,500.00	\$12,500	\$1,000.00	\$1,000	\$0.00	\$0	\$15,000
Div 4													
Div 5													
Div 6													
Div 7													
Div 8													
Div 9													
Div 10													
Div 11													
Div 12													
Div 13													
Div 14													
Div 15													
7		36" DIP	Depth	7			Depth of Cover	4					
8		36" Pipe Excavation - Earth (compacted volume)	CY	46200	\$0.75	\$34,650		\$0	\$3.00	\$138,600		\$0	\$173,250
9		36" Pipe Excavation - Trench Rock (compacted volume)	CY	15400		\$0		\$0		\$0	\$35.00	\$539,000	\$539,000
10		Trench Box	LF	39600		\$0	\$1.00	\$39,600		\$0		\$0	\$39,600
11		36" DIP Pressure Class 200	LF	31600	\$9.17	\$289,646	\$143.59	\$4,537,444	\$2.50	\$79,000		\$0	\$4,906,090
12		36" DIP Pressure Class 200 RJ	LF	8000	\$9.17	\$73,328	\$198.96	\$1,591,680	\$2.50	\$20,000		\$0	\$1,685,008
13		36" Pipe Bedding (compacted volume)	CY	8800	\$1.00	\$8,800	\$17.00	\$149,600	\$1.00	\$8,800		\$0	\$167,200
14		36" Pipe Backfill (compacted volume)	CY	42433	\$1.00	\$42,433		\$0	\$4.00	\$169,731		\$0	\$212,164
15		Import Backfill Materials (loose volume, assume 10% swell)	CY	0		\$0	\$13.00	\$0		\$0		\$0	\$0
16		Haul off Rock (assume 15% swell) - with Trench Rock	CY	17710		\$0		\$0		\$0		\$0	\$0
17		36" 90-degree Bend	EA	5	\$210.00	\$1,050	\$8,128.13	\$40,641	\$50.00	\$250		\$0	\$41,941
18		36" 45-degree Bend	EA	10	\$210.00	\$2,100	\$5,845.68	\$58,457	\$50.00	\$500		\$0	\$61,057
19		36" 22.5-degree Bend	EA	10	\$210.00	\$2,100	\$5,129.47	\$51,295	\$50.00	\$500		\$0	\$53,895
20		36" 11.25-degree Bend	EA	15	\$210.00	\$3,150	\$4,529.16	\$67,937	\$50.00	\$750		\$0	\$71,837
21			LS	1									\$0
22		Earthwork Calculations				\$0		\$0		\$0		\$0	\$0
23		Pipe Excavation - Total Compacted Volume	CY	61600		\$0		\$0		\$0		\$0	\$0
24		Rock - Total Compacted Volume (assume 25%)	CY	15400		\$0		\$0		\$0		\$0	\$0
25		Pipe Bedding - Total Compacted Volume	CY	8800		\$0		\$0		\$0		\$0	\$0
26		Pipe Backfill - Total Compacted Volume Needed	CY	42433		\$0		\$0		\$0		\$0	\$0
27		On-Site Backfill Material Available - Compacted Volume	CY	46200		\$0		\$0		\$0		\$0	\$0
28		Materials for Disposal - Compacted Volume	CY	3767	\$5.00	\$18,836		\$0	\$5.00	\$18,836		\$0	\$37,673
29													
30		Air Release Valve and Manhole (8 each)	LS	1	\$4,400.00	\$4,400	\$66,000.00	\$66,000	\$3,200.00	\$3,200	\$0.00	\$0	\$73,600
31													
Div 16													
32	16000	Electrical	LS	1		\$0		\$0		\$0	\$65,000.00	\$65,000	\$65,000
Div 17													
33	17000	Venturi Meter	LS	1	\$1,250.00	\$1,250	\$36,000.00	\$36,000	\$500.00	\$500		\$0	\$37,750
34	17000	Instrumentation	LS	1		\$0		\$0		\$0	\$7,500.00	\$7,500	\$7,500

WATERSHED DAM ASSESSMENT - (7194-002)
BARBER CREEK 06
OPINION OF PROBABLE CONSTRUCTION COST - CONCEPTUAL

02
 DECEMBER 2008

No.	Spec. Sect.	Description	Unit	Qty	Labor \$\$		Material \$\$		Equipment \$\$		Subcontractor \$\$		Total
					Unit	Total	Unit	Total	Unit	Total	Unit	Total	
02 - 36-inch Raw Water Line with Venturi Vault													
		Contingency	LS	5%		\$41,000		\$343,000		\$39,000		\$90,000	\$513,000
		Subtotals				\$862,243		\$7,238,254		\$818,567		\$1,901,400	\$10,820,463
		Sales Tax @		7.0%		\$506,700							
		Labor Burden @		30.0%		\$258,700							
		Bonds On Subs @		1.5%		\$28,500							
		Subtotal				\$11,614,363							
		Fee @		7.0%		\$813,000							
		Insurance & Bonds @		1.7%		\$211,300							
		Estimated Construction Cost				\$12,640,000							

Assumptions:
 DOES NOT include easements acquisitions, land acquisitions or mitigations required to construct the raw water transmission main
 Assumed 25% of the excavated material is rock

\$252 per LF (pipe only)
 \$319 per LF (pipe only)

WATERSHED DAM ASSESSMENT - (7194-002)

BARBER CREEK 06

OPINION OF PROBABLE CONSTRUCTION COST - CONCEPTUAL LEVEL

03

DECEMBER 2008

Table A-4

No.	Spec. Sect.	Description	Unit	Qty	Labor \$\$		Material \$\$		Equipment \$\$		Subcontractor \$\$		Total
					Unit	Total	Unit	Total	Unit	Total	Unit	Total	
03 - Reservoir Inlet Structure													
Div 1													
1	1000	General Conditions	LS	1		\$27,000		\$21,500		\$27,100		\$0	\$75,600
Div 2													
2	2200	Earth Work	LS	1	\$5,000.00	\$5,000	\$2,600.00	\$2,600	\$4,926.00	\$4,930	\$31,300.00	\$31,300	\$43,830
Div 3													
3	3250	Water Stop	LF	500	\$1.25	\$630	\$2.00	\$1,000		\$0		\$0	\$1,630
4	3300	Concrete	LS	1	\$122,409.00	\$122,410	\$223,638.00	\$223,640	\$37,750.00	\$37,750	\$0.00	\$0	\$383,800
Div 4													
Div 5													
Div 6													
Div 7													
Div 8													
Div 9													
Div 10													
Div 11													
5		Sluice Gates and Operators	EA	1	\$2,500.00	\$2,500	\$25,000.00	\$25,000	\$1,000.00	\$1,000		\$0	\$28,500
Div 12													
Div 13													
Div 14													
Div 15													
6	15062	Ductile Iron Pipe	LS	1	\$1,080.66	\$1,080	\$20,546.81	\$20,550	\$480.00	\$480	\$0.00	\$0	\$22,110
Div 16													
7	16000	Electrical	LS	1		\$0		\$0		\$0	\$60,000.00	\$60,000	\$60,000
Div 17													
8	17000	Instrumentation	LS	1		\$0		\$0		\$0	\$25,000.00	\$25,000	\$25,000
		Contingency	LS	5%		\$8,000		\$15,000		\$4,000		\$6,000	\$33,000
Subtotals						\$166,620		\$309,290		\$75,260		\$122,300	\$673,470
Sales Tax @				7.0%		\$21,700							
Labor Burden @				30.0%		\$50,000							
Bonds On Subs @				1.5%		\$1,800							
Subtotal						\$746,970							
Fee @				7.0%		\$52,300							
Insurance & Bonds @				1.7%		\$13,600							
Estimated Construction Cost						\$810,000							

Table A-5

Barber Creek 06**TOTAL PROJECT OPINION OF COST**

<u>Item . No.</u>	<u>Description of Work</u>	<u>Estimated Quantity</u>	<u>Unit</u>	<u>Unit Price</u>	<u>Amount</u>
1.	Mobilization and Demobilization	1	Job	<u>Lump Sum</u>	<u>\$812,636.34</u>
2.	Erosion & Sediment Control	1	Job	<u>Lump Sum</u>	<u>\$270,878.78</u>
3.	Control of Water	1	Job	<u>Lump Sum</u>	<u>\$406,318.17</u>
4.	Clearing	510	Ac	<u>2,000.00</u>	<u>\$1,020,000.00</u>
5.	Clearing & Grubbing	20	Ac	<u>\$3,500.00</u>	<u>\$70,000.00</u>
6.	Earth Fill	533,106	Cu-Yd	<u>\$2.50</u>	<u>\$1,332,765.00</u>
7.	Drain Fill	16,566	Cu-Yd	<u>\$50.00</u>	<u>\$828,300.00</u>
8.	Excavation, Common	61,912	Cu-Yd	<u>\$3.25</u>	<u>\$201,214.00</u>
9.	Riprap	19,014	Ton	<u>\$65.00</u>	<u>\$1,235,910.00</u>
10.	Permanent Turf Establishment	20	Ac	<u>\$2,000.00</u>	<u>\$40,000.00</u>
11.	Concrete, Class 4000 (reinforced)	10,023	Cu-Yd	<u>\$850.00</u>	<u>\$8,519,550.00</u>
12.	Concrete, Class 3000 (mass)	56	Cu-Yd	<u>\$400.00</u>	<u>\$22,400.00</u>
13.	24-Inch RCP	368	Feet	<u>\$350.00</u>	<u>\$128,800.00</u>
14.	Principal Spillway Riser	1	Lump Sum	<u>\$145,000.00</u>	<u>\$145,000.00</u>
<u>Dam Construction Cost Estimate</u>					<u>\$15,033,772.29</u>
15.	36-Inch Pipeline	1	Lump Sum	<u>\$12,640,000.00</u>	<u>\$12,640,000.00</u>
16.	Cascading Structure	1	Lump Sum	<u>\$810,000.00</u>	<u>\$810,000.00</u>

17.	Pumping Station (Including Raw Water Pumps and Access Road)	1	Lump Sum	\$8,480,000.00	\$8,480,000.00
	Pump Station and Pipeline Cost Estimate				\$21,930,000.00
18.	Land Acquisition	739	Ac	\$12,000.00	\$8,868,000.00
19.	Easement Acquisition	48	Ac	\$7,200.00	\$345,600.00
20.	Building Acquisition	17	Buildings	\$200,000	\$3,400,000.00
	Land Acquisition Cost Estimate				\$12,613,600.00
21.	Wetland	232	Credits	\$7,500.00	\$1,740,000.00
22.	Intermittent Stream	142,242	Credits	\$90.00	\$12,801,780.00
23.	Lower Perennial Stream	261,963	Credits	\$90.00	\$23,576,670.00
24.	Open Water	255	Credits	\$7,500.00	\$1,912,500.00
	Impacts and Overall Mitigation Cost Estimate				\$40,030,950
	Construction, Land Acquisition, Mitigation Estimate				\$89,608,322
	Suggested Project Estimate				\$90,000,000

The above suggested project cost estimate does not include contingencies or professional services. Professional services should be considered at not less than 15 percent of the suggested project cost estimate. Cost contingencies should be considered at not less than 25 percent of the suggest projet cost estimate. Prices are in 2008 U.S. Dollars