Water Supply Assessment for Lower Little Tallapoosa River No. 14

Carroll County, Georgia

Prepared for:

Georgia State Soil and Water Conservation Commission

Prepared by:

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December 18, 2007

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EXECUTIVE SUMMARY

The Georgia Soil and Water Conservation Commission (GSWCC), in partnership with the Natural Resources Conservation Service (NRCS) and the Georgia Environmental Protection Division (EPD) initiated a study to evaluate whether or not any of the existing watershed dams, designed and constructed under federal laws PL 544 and PL 566, could be modified to serve as water supply reservoirs. The evaluation process went through several iterations, the most recent of which can be found in the Finding Report dated December, 2007 on file with the GSWCC. The Finding Report identified 20 structures that had sufficient potential for relatively high yields with relatively small environmental and infrastructural impacts, when compared to the other projects evaluated. The selected twenty dams were further evaluated to identify project parameters.

The following report summarizes the evaluation of the Lower Little Tallapoosa River Structure Number 14, which is located in Carroll County, Georgia. For the purposes of this report, the existing normal pool will be raised to impound a water supply pool having a surface area of approximately 520 acres.

For convenience, the following summary lists the major findings of this evaluation. This summary should not be utilized as a separate document or in lieu of reading the entire report, including the Appendix.

- Approximately 686 acres of land will be impacted by the proposed reservoir and dam raising
- Approximately 18 structures will be impacted by the proposed reservoir and dam raising
- One county road will be impacted and the relocation of one county road.
- For the modeled conditions, the drought of record in the Lower Little Tallapoosa 14 Basin is the period 1999-2002. For a water supply storage of approximately 5.2 billion gallons and supplementation of natural reservoir inflow by pumped diversions (maximum 15 million gallons per day, mgd) from nearby Indian Creek, the safe yield of the reservoir is estimated to be 7.5 mgd.
- Approximately 33 acres of palustrine wetlands will be impacted by the proposed reservoir and dam raising
- Approximately 45 acres of lacustrine/palustrine open waters will be impacted by the proposed reservoir and dam raising
- Approximately 20,060 linear feet of lower perennial streams will be impacted by the proposed reservoir and dam raising
- Approximately 8,750 linear feet of intermittent streams will be impacted by the proposed reservoir and dam raising
- Review of available information did not indicate any cultural resources, protected species, primary or secondary trout streams, or 303(d) / 305(b) listed streams occurring within the maximum reservoir pool limits of Lower Little Tallapoosa River 14.
- Project cost is estimated in 2007 dollars at $112,000,000.

PREFACE

The results of the analyses presented herein are based upon United States Geological Survey (USGS) quadrangle maps and, therefore, should be utilized for planning purposes only. If the subject project is identified as having a possibility of progressing past this analysis, additional studies will be required. These studies will include but not be limited to detailed environmental evaluations, detailed yield analyses, preliminary engineering design, and detailed cost estimating. These additional studies will be required prior to beginning detailed design work and/or land acquisition. The level of study presented herein shall be considered as a screening tool to evaluate the proposed project relative to other projects. Until further studies are performed, actual yield and costs associated with the entire project cannot be readily determined.

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INTRODUCTION

The project team of Schnabel Engineering South, LLC (Schnabel), Jordan Jones and Golding (JJ&G), Joe Tanner and Associates, and the Law Office of William Thomas Craig were retained by the Georgia State Investment and Financing Commission as the agent for the Georgia Soil and Water Conservation Commission to evaluate 166 existing flood control structures. The subject structures were originally designed and constructed under Federal laws PL 544 and PL 566 to control storm water runoff (flooding) and collect sediment. The goal of this evaluation was to identify impoundments that could be enlarged to provide a relatively reliable water supply. The results of the evaluation were utilized to select twenty of the dams and reservoirs that had potential for relatively high yields with relatively small environmental and infrastructural impacts, when compared to the other projects evaluated. The selected twenty dams were further evaluated to identify project parameters. The additional evaluation included the following:

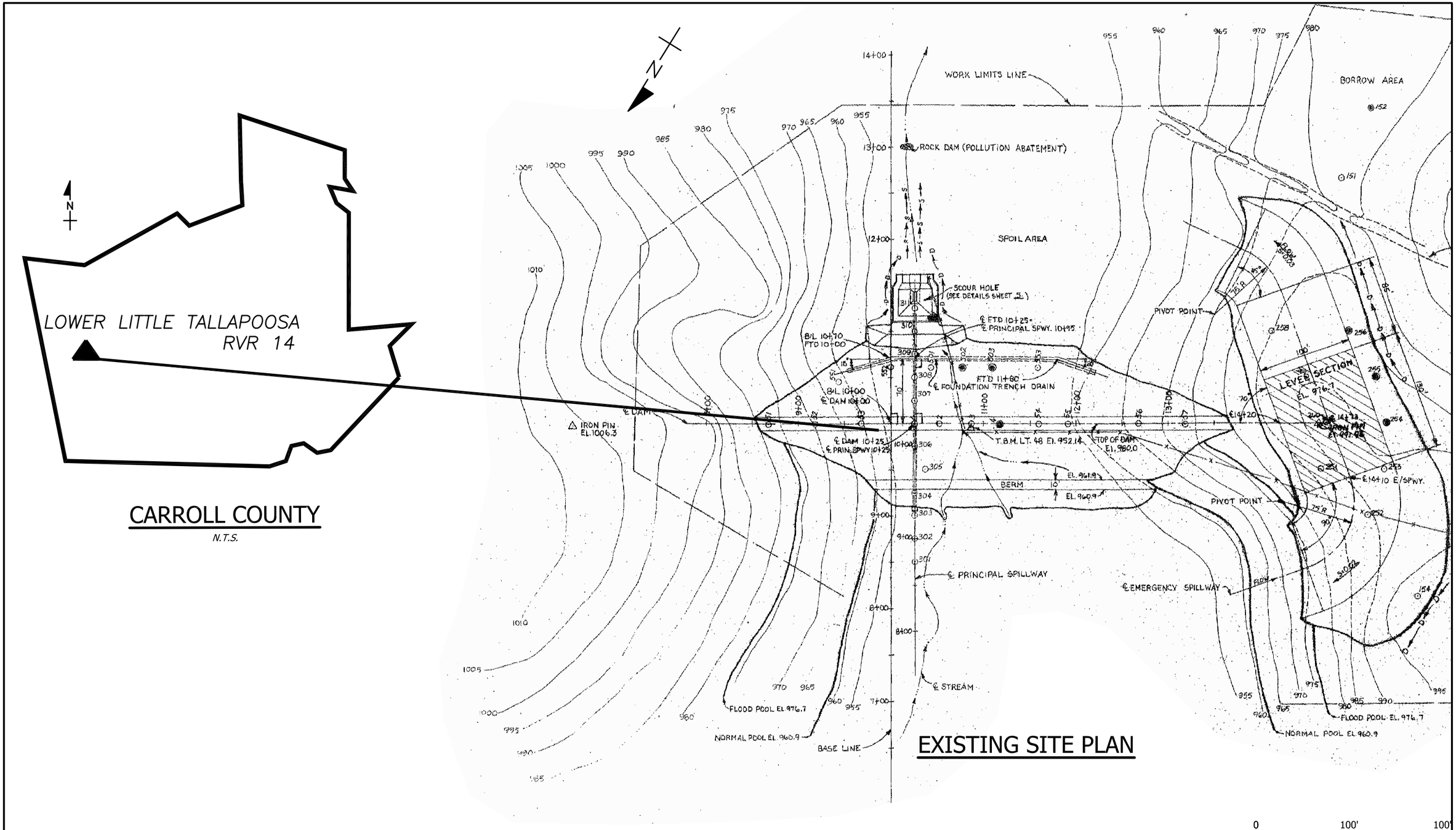
- More detailed yield analyses
- More detailed environmental evaluation
- Cost estimation of proposed modifications

The Lower Little Tallapoosa Watershed Dam Number 14 in Carroll County, Georgia was one of the structures selected for further evaluation.

BACKGROUND

The subject dam, Lower Little Tallapoosa River Watershed Dam Number 14 (LLTR # 14), is located approximately two miles northwest of Bowdon, Georgia in Carroll County. More specifically, the dam is located on Indian Branch approximately 1,000 feet upstream of the confluence of Indian Branch and Indian Creek.

The existing earth fill dam was designed in 1977 and constructed in 1979. As designed, the dam had a crest elevation of 980.0 feet and impounded a reservoir that had a surface area of approximately 37 acres at a normal pool elevation of 960.9 feet. The crest of the emergency spillway was designed to be at elevation 976.7 feet. Figure 1 shows the location of the subject dam within the county as well as a plan view of the existing embankment and emergency spillway. This dam was designed as a flood-control structure. According to the Soil Conservation Service (SCS), now known as the Natural Resources Conservation Service (NRCS), Dam Inventory sheet, the dam was originally designed and constructed as a Class ‘A’ or low-hazard dam. The state Safe Dams program classifies the existing dam as a Category 2 structure. When designed, the emergency spillway (now referred to as an auxiliary spillway) had a four percent chance of operating in any given year. This results in the auxiliary spillway operating during storm events equal to and greater than the 25 year storm event. With the exception of engineering, land acquisition, and project administration, the dam was completed for a cost of approximately $153,000.



	Schnabel Engineering	NRCS WATERSHED DAM ASSESSMENT LOWER LITTLE TALLAPOOSA NO. 14	SITE LOCATION MAP PROJECT NO. 07170030.01 FIGURE 1

NEEDS AND DEMAND EVALUATION

Population projections through the year 2025 were obtained from the Carroll County Comprehensive Plan (adopted in December, 2004). Projections to 2057 were extrapolated based on the assumption of the same constant growth rate that was shown in the Comprehensive Plan. These projections can be seen in Table 1.

Table 1
Population Projection

Year	Population Projection
2000	87,268
2005	101,844
2010	114,551
2015	128,844
2020	144,920
2025	163,002
2030*	183,340
2035*	206,216
2040*	231,946
2045*	260,886
2050*	293,438
2055*	330,051
2057*	346,524

Data Source: from Planning Works, LLC in Carroll County Comprehensive Plan
**Population Calculated based on yearly % growth from 2005-2025*

Water demand projections were calculated based on population projections and water withdrawal data for Carroll County in 2000. According to the US Census, the population of Carroll County was 87,268 in 2000, while the water withdrawal was 11 million gallons per day (MGD) based on the document "Water Use in Georgia by County for 2000", (Information Circular 106, Julia Fanning, USGS, Atlanta, 2003). The Carroll County Water Authority currently holds a surface water withdrawal permit from the HC Seaton Reservoir for 8.0 MGD. Municipalities within the county also hold the following surface water withdrawal permits: the City of Carrolton (12 MGD from the Little Tallapoosa River), the City of Villa Rica (1.5 MGD from Lake Paradise and Cowens Lake), and the City of Bowden (1.0 MGD from Lake Tysinger, and 0.36 MGD from Indian Creek). In addition to the surface water permits, the Carroll County Water Authority and the City of Villa Rica hold groundwater withdrawal permits for 0.75 MGD and 0.125 MGD respectively. All totaled, water withdrawal permitted for public use in Carroll County is 23.7 MGD (all numbers are reported in permitted monthly average).

The overall usage was calculated to be 130 gallons per day (gpd) per person. This number was used as a constant through 2057 to create water withdrawal projections. The water withdrawal projection for 2057 was calculated to be approximately 45 MGD. This figure includes all unaccounted for water (UAW), and the assumption that industrial usage would increase with the increase in Carroll County population. Water withdrawal projections are shown in Table 2.

Table 2
Water Withdrawal Projection

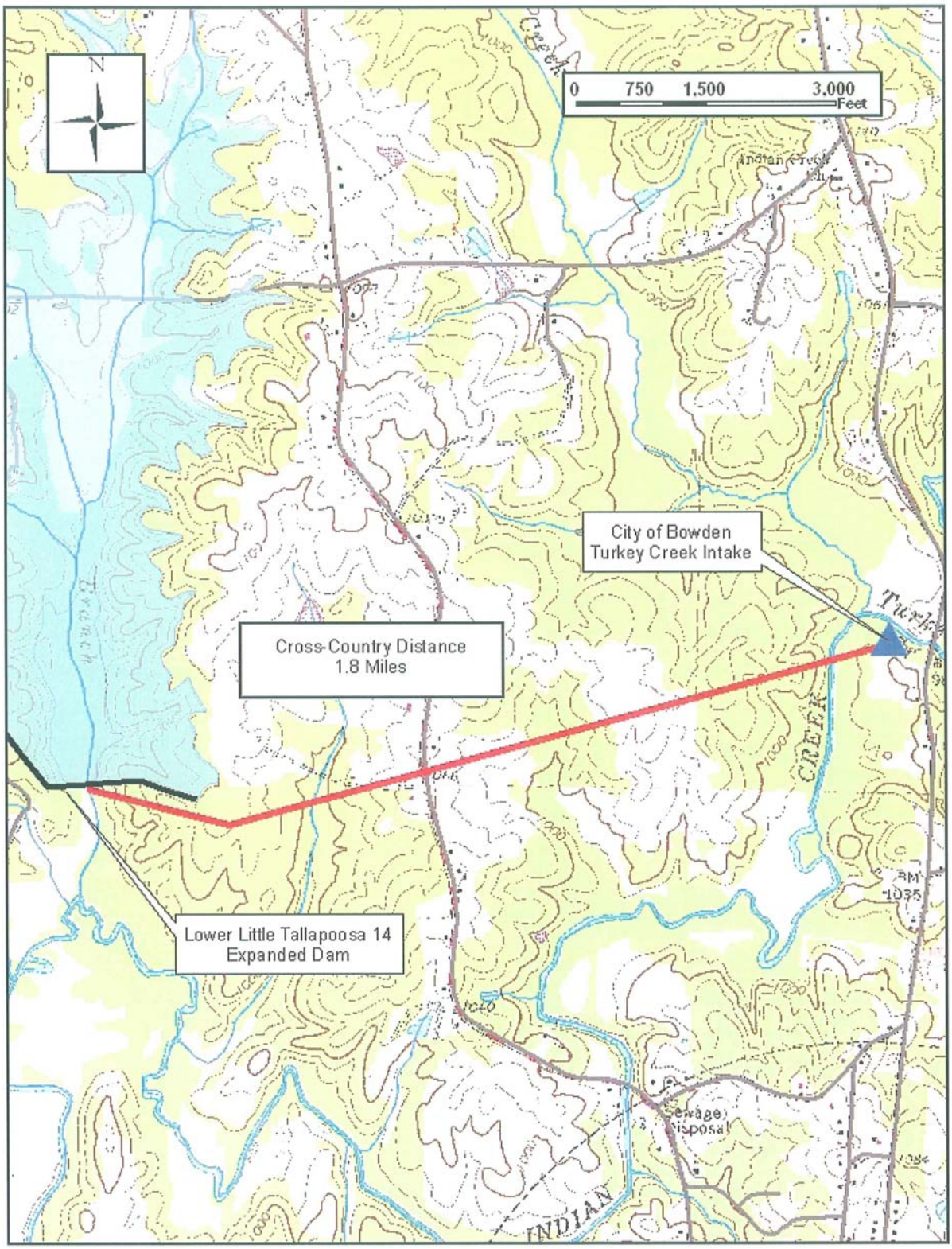
Year	Water Withdrawal Projection (MGD)
2000	11
2005	13
2010	15
2015	17
2020	19
2025	21
2030	24
2035	27
2040	30
2045	34
2050	38
2055	43
2057	45

Proximity to Surface Water Intakes

Based on the GIS database developed for this project, there is no surface water intake structure downstream of the dam within the State of Georgia. There may be an intake in Alabama, but these were not investigated or located for this project. The Alabama state line is only approximately 2.1 miles directly west of the dam; however, the stream distance to the state line is approximately 8.8 miles, south-southwest of the dam. This mileage includes 0.25 miles from the dam to Indian Creek, and then 0.1 miles from the Indian Creek confluence with Little Tallapoosa River to the state line. The remainder of the stream distance is in Indian Creek.

The nearest surface water intake in Georgia to the dam is located approximately 1.8 miles to the east. This is an intake operated by the City of Bowdon located on Turkey Creek. The following figure illustrates the location of the nearest surface water intake location to Lower Little Tallapoosa River 14.

Figure 2
Distance to Nearest Intake



ENGINEERING FACTORS

Proposed Dam

The proposed dam, which will incorporate the existing dam, will have a crest elevation of 1040 feet, an auxiliary spillway elevation of 1030 feet, and a water supply pool elevation of 1028 feet. The proposed dam will impound a reservoir that has a surface area of approximately 526 acres and storage volume of approximately 5,220 million gallons (MG) at the water supply pool elevation. A plan view of the proposed reservoir is shown in Figure 3.

Several engineering assumptions were made pertaining to spillway configuration. The spillway system for the proposed dam was assumed to consist of a principal spillway in the form of a 2'-6" by 7'-6" interior dimension reinforced concrete riser with a 30-inch diameter reinforced concrete low-level outlet pipe and an auxiliary spillway in the form of a 220-foot wide reinforced concrete chute spillway with ogee crest. The intent of the proposed principal spillway is to approximate the flows that are being discharged by the current spillway system during the two through 100-year storm events. The size of the auxiliary spillway was approximated by estimating the peak inflow that would occur during the Probable Maximum Precipitation (PMP) event and computing the spillway width that would be required to pass the estimated inflow with a given amount of hydraulic head. The available hydraulic head was determined by comparing the drainage basin area to lake surface area. The structures that had a drainage basin area to lake surface area ratio equal to or in excess of ten were allotted 15 feet of hydraulic head to pass the PMP inflows, while the structures that had a ratio of less than ten where allotted ten feet of hydraulic head to pass the PMP inflows. The assumption that the dam would be required to pass the inflow resulting from the PMP storm event is based on the history of the Georgia Department of Natural Resources Environmental Protection Division Safe Dams Program (Safe Dams) reviewing plans for water supply reservoir dams regardless of classification. As such, the dam would generally be required to comply with the engineering guidelines established by Safe Dams. Based upon the height of the dam (approximately 100 feet), the dam would be required to store and/or pass the inflows from the full PMP event safely. Additionally, the proposed dam would have a relatively high likelihood of being classified as high-hazard or Class 'C' by the NRCS, as well as Safe Dams.

The proposed dam and flood pool will:

- Impact 18 structures
- Require the purchase of 629 acres from 32 parcels
- Require the purchase of 57 acres of easement area for state required buffer
- Impact one local/county road

Figure 4 displays the proposed reservoir area as well as the buffer and affected parcels. The 18 affected structures were identified from aerial photographs. The types of structures were not identified on the ground and could be houses, barns, trailers, etc. A more detailed ground survey will be required to determine the type of each structure and the corresponding purchase price of each structure.

Figure 3
Proposed Reservoir Area Map

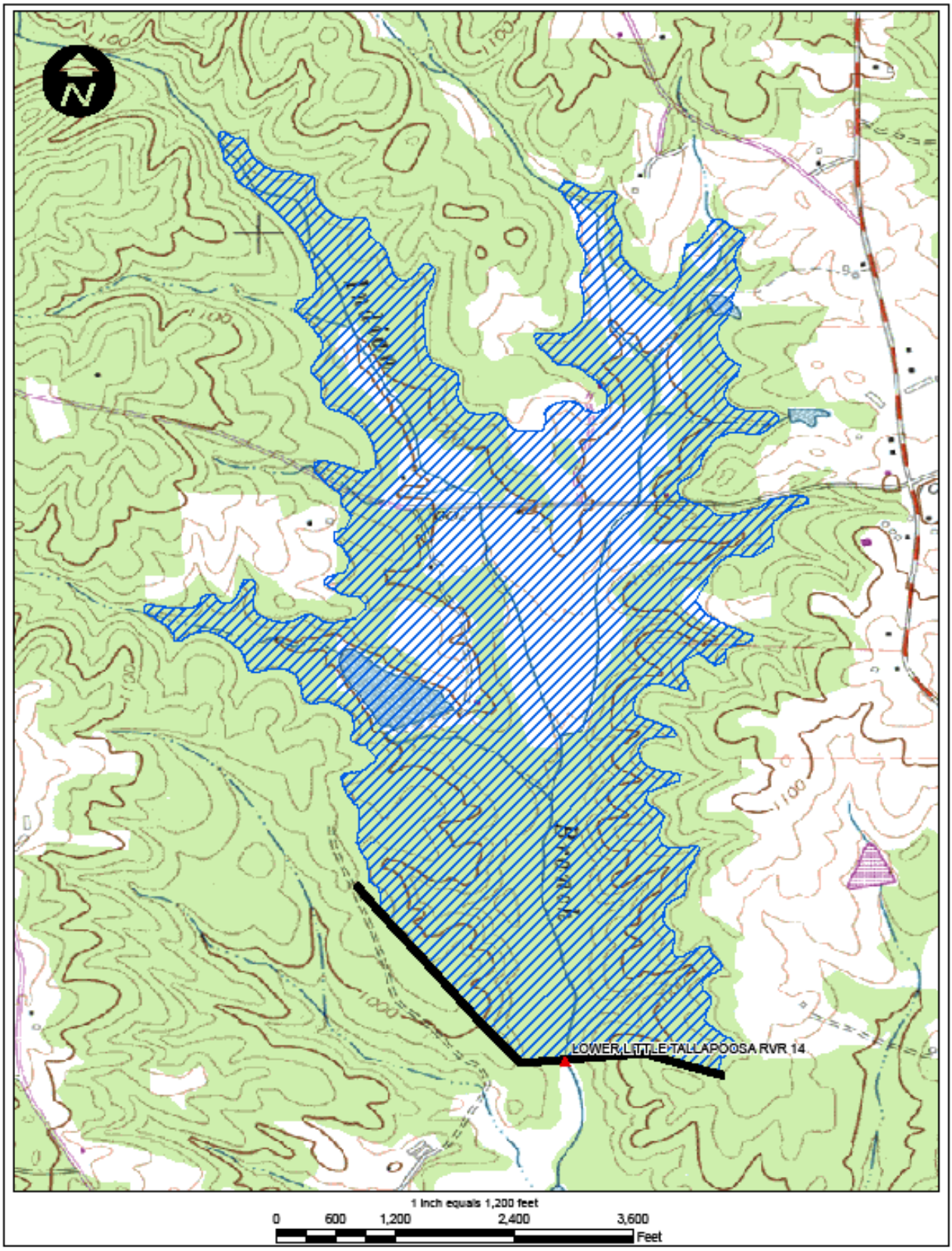
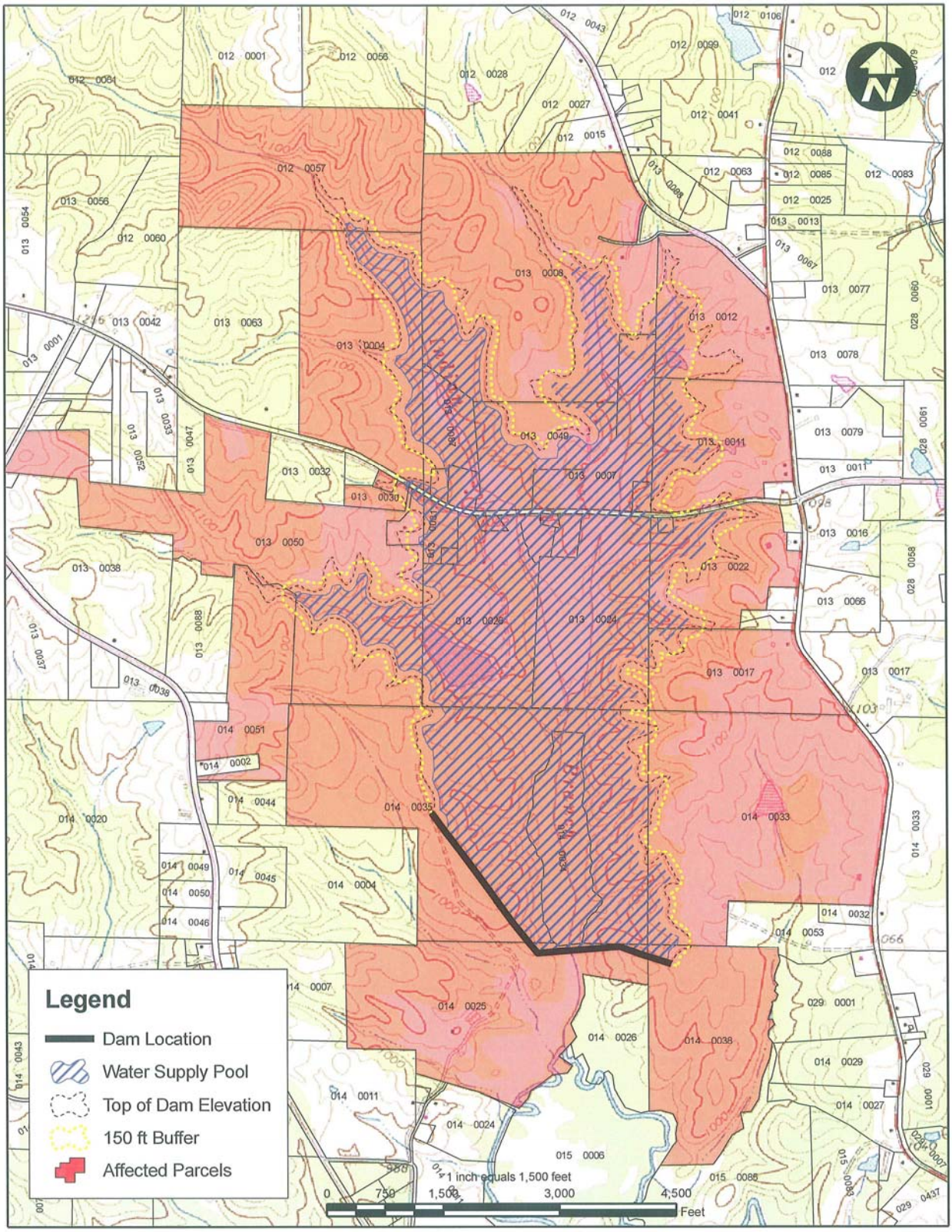


Figure 4
Land Acquisition and Buffer Areas



SAFE YIELD ANALYSIS

Definition

Reservoir safe yield is generally defined as the reliable withdrawal rate of water with acceptable quality that can be provided by reservoir storage through the critical drought period. The critical drought period in the State of Georgia is defined as the drought of record and in any given drainage basin can vary depending on reservoir size and other factors. This study was based on the critical drought period from 1999-2002; however, the current drought could possibly exceed the existing drought of record. If this were to occur, the computed yields detailed herein would be reduced. Safe yield in this study was simulated using a constant average annual demand. The justification for this is that while total water demands after declaration of a drought condition are usually less than normal, this situation is typically offset by higher than average demands prior to declaration of the drought condition. Safe yield is dependent upon the storage and hydrologic (rainfall/runoff/evaporation) characteristics of the source and source facilities, the selected critical drought, upstream and downstream permitted withdrawals, and the minimum in-stream flow requirements.

The proposed reservoir is a "pumped-storage" reservoir, where natural inflow into the reservoir is supplemented with pumped diversions from a nearby larger stream or river. Water is pumped from a larger river when runoff is plentiful, and is stored in the reservoir for times of drought. Pumped diversions increase safe yield, and generally result in fewer environmental impacts compared with reservoirs on main-stem rivers.

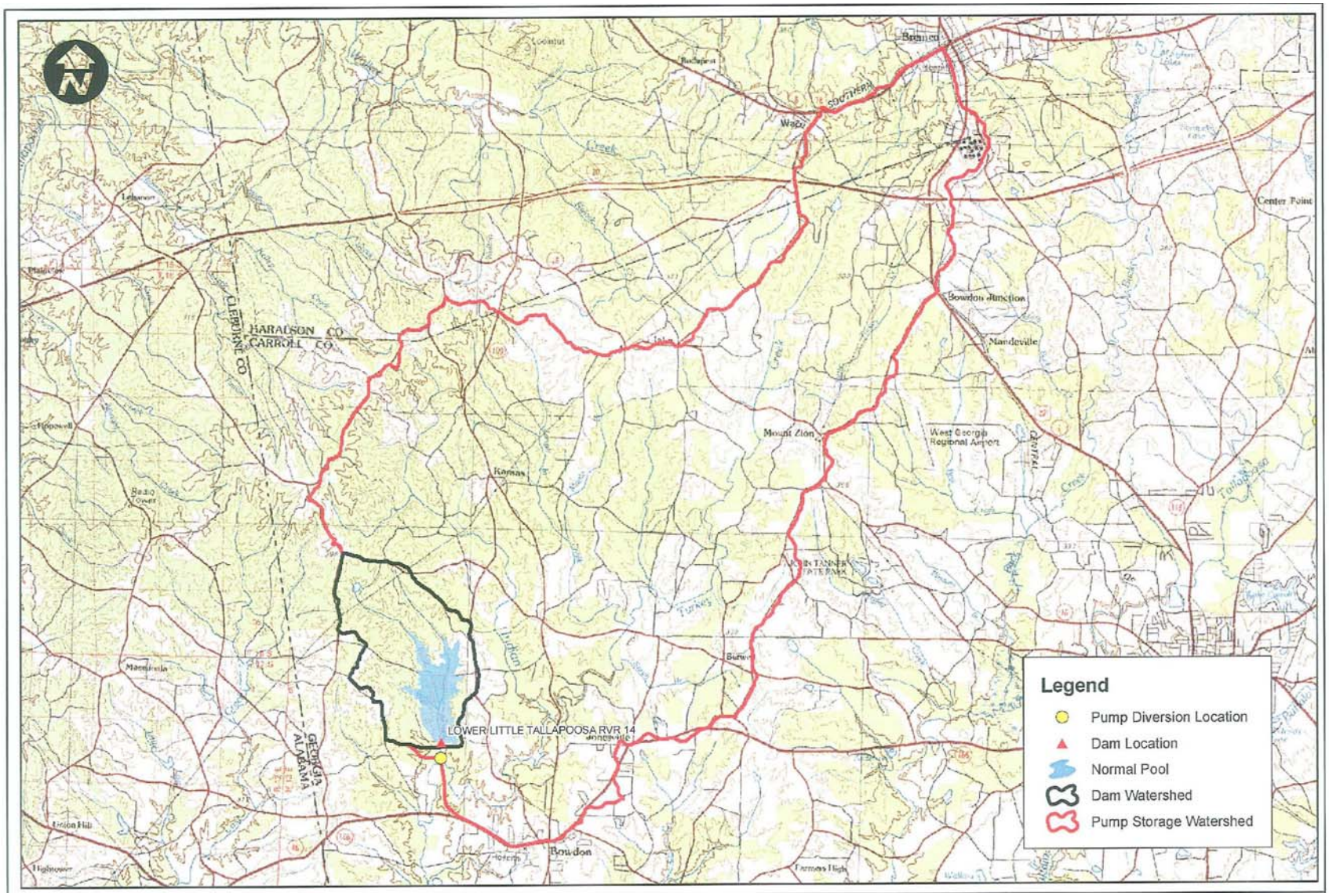
Analysis Method

The Little Tallapoosa River below Bowden gage (USGS 02413210) was selected for use in this analysis; however, its record period only extends from December 1999 to September 2004. Therefore, a correlation of the Bowden gage with the Tallapoosa River Near Heflin, AL gage (USGS 02412000) was performed, and regression-based adjustment was applied to the Heflin gage flows (Figure A-1, Appendix) to lengthen the simulation period. The adjusted flows from the Heflin, AL gage were then used to simulate stream flows in the Indian Creek and Indian Branch basins. The record period for the Heflin gage (adjusted) extends from July 1952 to present and includes three major droughts (1954-57, 1986-88, 1999-2002), plus the current drought. The diversion pump station was assumed to be located just upstream of the confluence of Indian Branch with Indian Creek. The straight line pipe distance between the dam and diversion location was estimated at 0.3 mile. The following drainage areas were used in the analysis:

- Dam Site (Indian Branch): 4.13 mi^2
- Diversion (Indian Creek) 55 mi^2

The pumped diversion location and watershed is shown in Figure 5. The maximum estimated pool level at top of dam was selected during the initial screening phase based on USGS topographic mapping. From that level, a freeboard allowance of 10 feet

Figure 5
Watershed Location Map

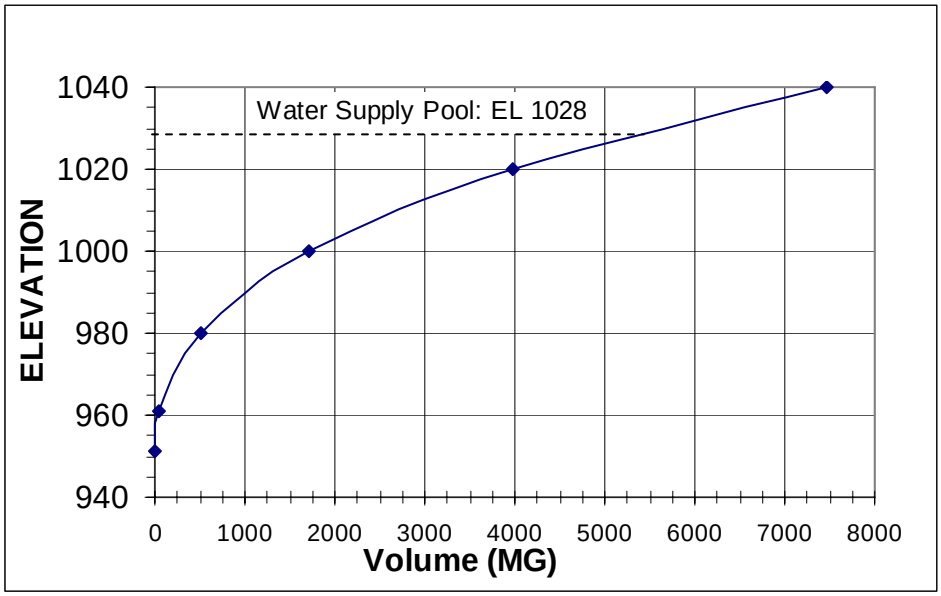


between the top of dam and the auxiliary spillway was incorporated to pass the spillway design flood (assumed to be the probable maximum flood). Additional depth to maintain existing flood storage volume (1162 Ac-ft, or 379 MG) was subtracted from the auxiliary spillway elevation to compute the water supply pool elevation used in the analysis of safe yield. Note that more detailed topographic mapping would be needed to approximate the safe yield of the proposed dam more closely. Table 3 summarizes the various reservoir elevations and associated approximate storage volumes. Calculation of stage-area and stage-storage curves is presented as Figure A-2 in the Appendix. Figure 6 below is the stage-storage curve for the reservoir.

Table 3
Summary of Reservoir Data

Stage	Elevation	Volume (Million Gallons)
Maximum Pool (Top of Dam)	1040	7,500
Flood Pool (Auxiliary Spillway Crest)	1030	5,600
Water Supply Pool	1028	5,200

Figure 6
Stage-Storage Curve



A reservoir operations model was developed to incorporate daily gage data from the selected USGS gage and reservoir shape parameters for estimation of evaporation. The following assumptions were incorporated into the analysis for the estimation of safe yield:

Assumptions

1. Dead storage of 20% of gross reservoir storage was incorporated to allow for sediment storage and poor water quality in lower reservoir strata.
2. Usable water supply storage was assumed to be the water supply pool storage (calculated as noted above) less dead storage.
3. Pump station diversions were assumed to be from Indian Creek at the location previously described. Diversions were assumed to occur whenever the reservoir level fell below full water supply pool. Pumped diversions were assumed to be bounded by pumping capacity and by flow restrictions on Indian Creek (noted below).
4. A minimum in-stream flow (MIF) of 30% of the average annual flow (AAF) at the diversion pump station (Indian Creek) was used.
5. No downstream permitted withdrawals were identified; therefore, no additional non-depletable flows were simulated.
6. Upstream withdrawals from Indian Creek by the Town of Bowden would reduce available flow in the stream. The model incorporated the upstream withdrawal with the following characteristics:
    - Withdrawal: 0.36 mgd
    - Drainage Area: 39.9 mi^2
    - MIF none
7. For the dam site, minimum in-stream flow of 30/60/40 percent average annual flow (AAF) was used. This MIF applies as follows: 30% AAF for July through November; 60% AAF for January through April; and 40% AAF for May, June and December.
8. Return flow from wastewater discharges or septic systems was not considered in the analysis.
9. Evaporation loss was based upon net historical evaporation rates (maximum average day) for each month as recorded at Allatoona Dam (Station No. 181) in Bartow County. Lake evaporation was assumed to be equal to 70% of pan evaporation during each month. Surface area was approximated by a regression equation relating storage to surface area (Figure A-3, Appendix).
10. Stream flow data from the USGS gage was applied in direct proportion of drainage areas to simulate flow into the reservoir and at the diversion location.
11. Total seepage losses would be less than the MIF requirements and, therefore, did not need to be separately considered.
12. Safe yield is that quantity of water that can be provided to meet water demands during the critical drought period.

The attainable safe yield during the analyzed period was found by iteration of the daily mass balance equation:

Ending Storage = (Beginning Storage) + (Natural Inflow) + (Pumped Inflow) – (Water Supply) – (Evaporation) – (MIF)

The trial safe yield value was varied until the reservoir level reached the dead storage value, and recovery of the reservoir was computed.

RESULTS

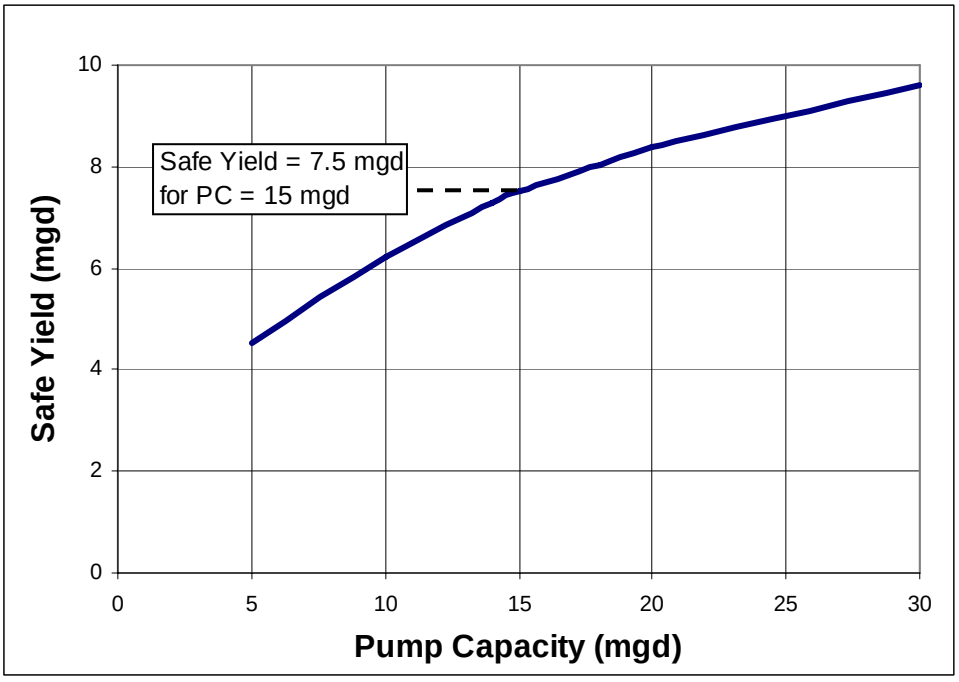
Incorporating the above assumptions, the estimated safe yield of the site was computed. The results of the safe yield analysis are presented in Table 4 and Figure 7. It should be noted that these estimated safe yield values are based on USGS topographic mapping. The estimates could vary significantly based on more detailed mapping, which would be required as part of a final safe yield analysis. The table below presents the estimated safe yield and refill time for a range of pump capacities. We have assumed a refill time of 4 to 5 years is the maximum refill duration for selection of pump capacity (PC).

Table 4
Safe Yield Summary

Pump Capacity (MGD)	Estimated Safe Yield (mgd)	Refill Time* (years)
5	4.5	15
10	6.2	10
15	7.5	5
20	8.4	4
30	9.6	4

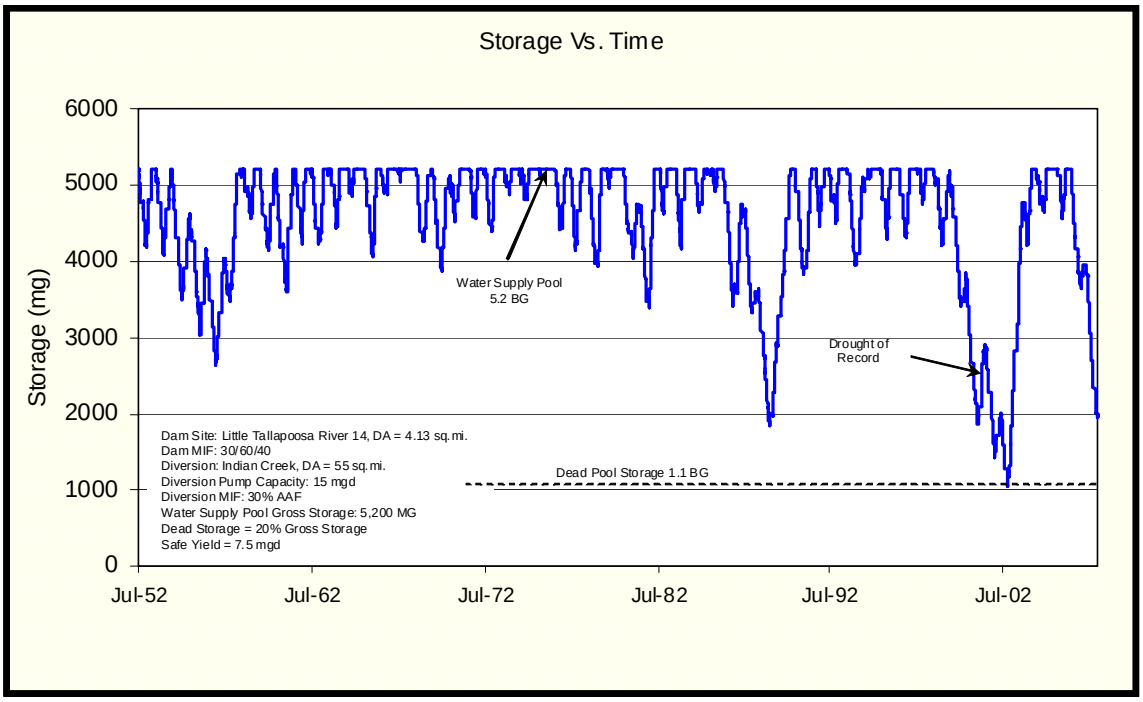
*Refill time is the time from start of drawdown until complete refill to water supply pool

Figure 7
Estimated Safe Yield vs. Pump Capacity



As presented in Figure 7, there is diminishing return (safe yield) with increasing pump capacity (reflecting pump station and pipeline cost). For the purposes of this analysis, an estimated economical safe yield & pump capacity combination were selected from the above graph. The estimated safe yield for this project is approximately 7.5 mgd for a pump capacity of 15 mgd. These values were used to size and cost out the diversion facilities detailed later in this report. The variation of reservoir elevation over time for the above assumed safe yield and pump capacity is reflected in Figure 8.

Figure 8
Reservoir Elevation vs. Time



ENVIRONMENTAL CONSIDERATIONS

Preliminary Studies

To evaluate the potential environmental impacts, permitting and compensatory mitigation associated with Lower Little Tallapoosa River 14, preliminary ecological studies were conducted by JJG. These studies consisted of a desktop survey and wetland approximation field surveys to estimate wetlands and streams occurring within the project area. While this evaluation is not sufficient for Clean Water Act Section 404 permitting, field surveys add increased confidence to the desktop evaluation. All estimates of jurisdictional waters, permitting requirements, and compensatory mitigation requirements/cost estimates presented herein are very general and preliminary in nature. Detailed studies would be necessary to definitively determine permitting requirements.

Prior to conducting field surveys, desktop evaluations were performed with available data resources including the U.S. Geological Survey 7.5-minute topographic maps and U.S. Fish and Wildlife Service (USFWS) National Wetland Inventory (NWI) maps. JJG ecologists then performed a reconnaissance-level site visit to Lower Little Tallapoosa River 14 site to verify and supplement the desktop evaluation. Subsequent to field surveys, observations were transcribed into an ArcView GIS database for analysis. Preliminary estimates of jurisdictional waters (i.e., wetlands, streams, open waters) occurring within the Lower Little Tallapoosa River 14 project area are provided below.

Wetlands

The *Classification of Wetlands and Deepwater Habitats of the United States* (Cowardin Classification System) defines the Palustrine System as all nontidal wetlands dominated by trees, shrubs, persistent emergent vegetation, emergent mosses or lichens, and all such wetlands that occur in tidal areas where salinity is less than 0.5 percent. It also includes wetlands lacking such vegetation, but with all of the following four characteristics: 1) area less than 20-acres; 2) the lack of active wave-formed or bedrock shoreline; 3) water depth in the deepest part of basin less than 6.6 feet at low water; and 4) salinity due to ocean-derived salts less than 0.5 percent.

The Lacustrine System includes wetlands and deepwater habitats with all of the following characteristics: 1) situated in a topographic depression or a dammed river channel; 2) lacking trees, shrubs, persistent emergent vegetation, emergent mosses or lichens with greater than 30-percent areal coverage; and 3) total area exceeds 20 acres. Wetlands and deepwater habitats less than 20-acres are also included in this system if an active wave-formed or bedrock shoreline feature makes up all or part of the boundary, or if the water depth in the deepest part of the basin exceeds 6.6 feet at low water.

Office and field reviews determined that approximately 33 acres of palustrine wetlands and approximately 45 acres of lacustrine/palustrine open waters exist within the Lower Little Tallapoosa River 14 project area. These systems are primarily associated with Indian Branch and unnamed tributaries within the proposed reservoir pool limits. Cowardin classifications of the wetland systems range from palustrine forested to

palustrine emergent with hydrologic regimes ranging from saturated to seasonally flooded.

Streams

The Cowardin Classification System defines lower perennial streams as low gradient streams with slow water velocities and substrates comprised mainly of sand and mud. Intermittent streams are defined as streams flowing for only part of the year. When water is not flowing, it may remain in isolated pools or surface water may be absent. Ephemeral streams flow only in direct response to precipitation and do not receive groundwater contributions.

Office and field reviews indicate that approximately 20,060 linear feet of lower perennial streams and approximately 8,750 linear feet of intermittent streams are located within the maximum reservoir pool limits of Lower Little Tallapoosa River 14. Ephemeral streams were not identified due to the preliminary nature of the studies. Refer to Figure 9 for locations of these jurisdictional features.

Cultural Resources

Review of existing cultural resources information did not indicate any identified cultural resources within the maximum reservoir pool limits of Lower Little Tallapoosa River 14. It should be noted that the absence of recorded Cultural Resources does not mean that they do not exist; in fact, a Phase I Cultural Resources Survey (conducted to the standards of Section 106 of the National Historic Preservation Act) would be required to determine the presence or absence of Cultural Resources as part of permitting for any proposed reservoir project.

Threatened and Endangered Species

Review of existing threatened and endangered species information did not identify any known occurrences of protected species within the maximum reservoir pool limits of Lower Little Tallapoosa River 14. Six protected species are known from Carroll County, Georgia and include three faunal and three floral species. Refer to Table 5 for a summary of protected species located in Carroll County and potential habitat for these species within the maximum reservoir pool limits.

Figure 9
Jurisdictional Areas Location Map

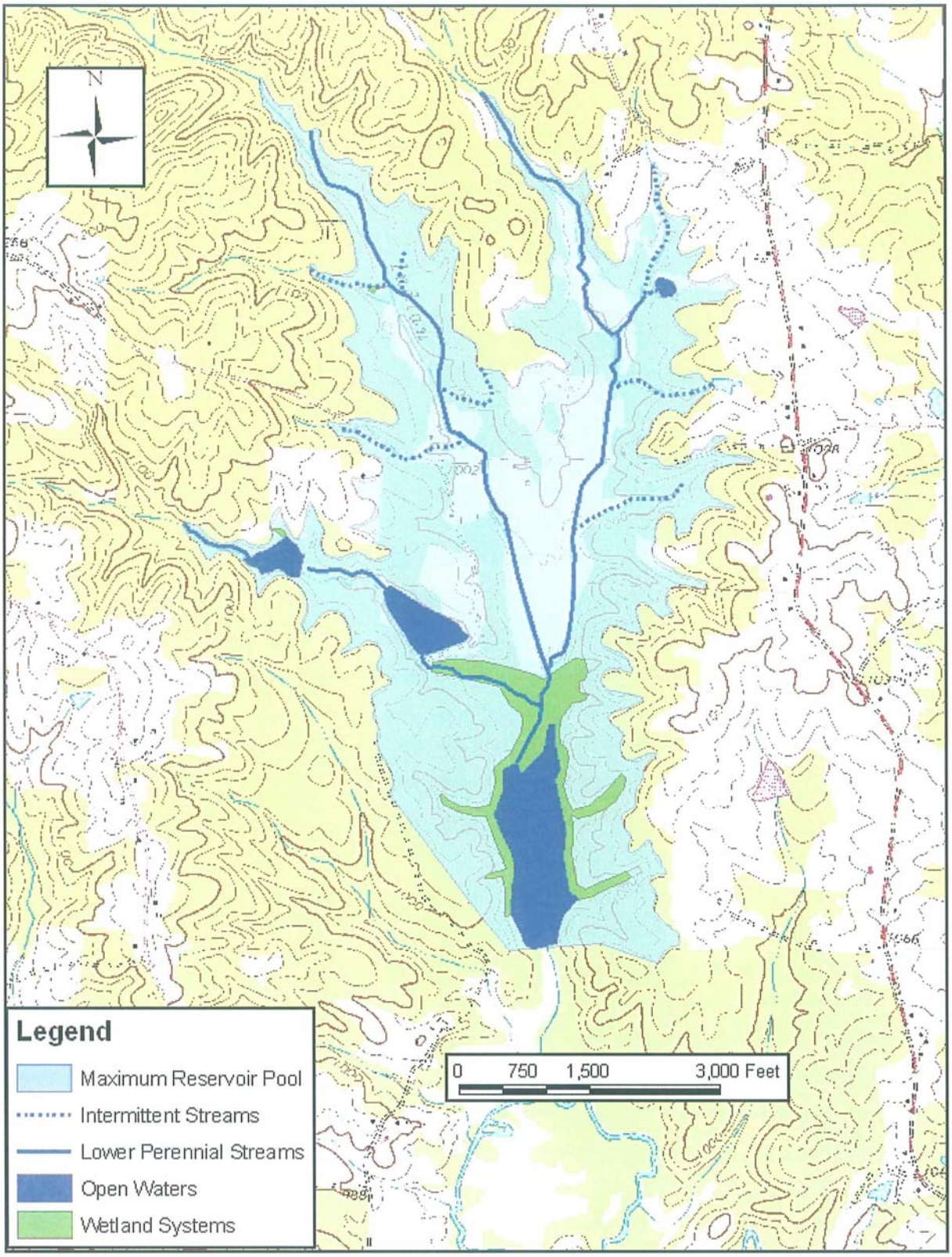


Table 5
Summary of Protected Species for Carroll County, Georgia

Scientific Name	Vernacular Name	Federal Status	State Status	Habitat Present (Yes/No)	Preferred Habitat
Faunal species					
Cyprinella callitaenia	bluestripe shiner	NA	T	No	flowing areas in large alluvial rivers with open, sand or rock bottomed channels with little or no aquatic vegetation
Notropis hypsilepis	highscale shiner	NA	T	Yes	flowing areas of small to medium streams over sand or bedrock substrates
Fundulus bifax	stippled studfish	NA	E	Yes	pools, stream margins, and backwaters over sandy or rocky substrates within the Coosa and Tallapoosa River systems
Floral species					
Platanthera integrilabia	white fringeless orchid	CS	T	Yes	red maple-blackgum swamps; sandy, damp stream margins; seepy, rocky, thinly vegetated slopes
Schisandra glabra	bay star-vine	NA	T	Yes	twining in subcanopy and understory tress/shrubs in rich alluvial woods
Waldsteinia lobata	Piedmont barren strawberry	NA	T	No	rocky woods along streams with mountain laurel

T= threatened, E= endangered, CS= candidate species, NA= not applicable

Trout Streams

Review of available resources did not indicate any primary or secondary trout streams within the maximum reservoir pool limits of Lower Little Tallapoosa River 14.

303(d) and 305(b) Listed Streams

Review of available resources did not indicate any 303(d) or 305(b) listed streams within the maximum reservoir pool limits of Lower Little Tallapoosa River 14.

Section 404/401 Permitting

The U.S. Army Corps of Engineers (USACE) regulates the discharge of dredged or fill material into the Nation's Waters under Section 404 of the Clean Water Act. Construction of an impoundment and flooding jurisdictional streams/wetlands is regulated by the USACE. Two types of permits are available through the USACE: Nationwide and Individual Permits. Nationwide Permits (NWP) have been established previously by the Chief of Engineers for projects that have minimal cumulative impacts to the Nation's Waters. Examples of the most commonly used NWPs include site development, minor road crossings, maintenance activities, and utility line discharges. Specific criteria and conditions were established that must be satisfied prior to obtaining authorization of a NWP from the USACE. In addition, the Savannah District of the USACE issued Final Nationwide Permit Regional Conditions effective May 11, 2007.

Individual Permits (IP) are required for projects having more than minimal cumulative adverse impacts on the Nation's waters. The development of a water supply reservoir would typically require an IP. IP's involve significantly more information, documentation, and coordination with regulatory agencies and are considerably more difficult to acquire than a NWP. Prior to coordination with the USACE regarding the construction of an impoundment, required information would consist of, but not be limited to, the following information:

- Justification of Purpose and Need for the project
- Alternatives analysis of other water supply options evaluated to meet the need
- Wetland delineation with surveyed boundaries of USACE jurisdictional waters
- Phase I cultural resources and protected species surveys
- Detailed description of proposed project and proposed impacts to jurisdictional waters
- Detailed analysis of flow releases documented with population analysis and system modeling
- Avoidance and minimization of jurisdictional waters analysis
- Identification of adjacent property owners
- Development of a conceptual compensatory mitigation plan

Following completion of these items, a complex project meeting would typically be scheduled with the USACE Northern Area Section Office (Morrow, GA) to present the proposed project. Subsequent to the meeting, and if a project is tentatively accepted by the regulatory agencies, formal application and preparation of an IP would start. Following submittal of an IP, the application must be advertised for public comment. The USACE prepares the public notice, which includes detailed applicant information such as site location, proposed impacts, cultural resources, protected species, and proposed mitigation. The public notice would be advertised for 30 days and is also submitted to regulatory agencies including the Environmental Protection Agency (EPA) and USFWS, adjacent property owners, and to the USACE general mailing list. Applicants will be required to respond to inquiries received during the public notice process. Public hearings could be required if substantial adverse comments are received from the coordinating agencies or the public. Additional information and permitting required would consist of a Section 401 Water Quality Certification from the Georgia Environmental Protection Division (EPD). This certification must be issued for an IP to be valid. Depending on the level of impacts associated with the proposed reservoir, an Environmental Assessment or Environmental Impact Statement could be required by the USACE as well. Based on previous project experience, the level of controversy and environmental issues raised during agency and public review, a typical new reservoir project may require permitting times of 5 years or more.

The expansion of an existing reservoir could potentially facilitate the Section 404 permitting process when compared to the construction of a new impoundment. This is especially true for issues such as alternatives analysis, avoidance and minimization, and aquatic organism passage in that many or most potential impacts have already occurred. However, the steps of the overall Section 404 permitting process would still need to be followed, and historically reservoirs have encountered significant regulatory and public challenges, regardless of the presence/absence of an existing impoundment.

Compensatory Mitigation

To determine the amount mitigation potentially required for jurisdictional impacts within the Lower Little Tallapoosa River 14, the USACE's Standard Operating Procedure (SOP) for Compensatory Mitigation (March 2004) was utilized. The SOP uses a series of factors such as location, type, existing condition, type of impact, etc. to generate a multiplying "factor." That factor is then multiplied by the impact area (acreage or linear footage) to calculate the required mitigation credits. To determine an average factor for jurisdictional areas associated with the Lower Little Tallapoosa River 14, various conditions observed during the field surveys were utilized. *However, it is imperative to note that this document only serves as a guideline if impacts do not exceed 5,000 linear feet of stream or ten acres of wetland impacts.* Potential impacts for the Lower Little Tallapoosa River 14 would significantly exceed this threshold and actual compensatory mitigation requirements would likely be substantially different from SOP estimates. Currently, the USACE Savannah District Office is developing a new SOP for large-scale projects focused on reservoirs. It is anticipated that this SOP would be issued mid-2008.

Utilizing the 2004 SOP and the approximated acreage and linear feet of jurisdictional waters located within the Lower Little Tallapoosa 14 project area, an estimate of

compensatory mitigation credits can be determined. Multiplying factors used for this analysis include: 6.7 for wetland systems, 5.7 for open waters, 12.7 for lower perennial streams, and 7.6 for intermittent streams. This factor was then multiplied by the acreage/ linear footage to determine an estimated number of mitigation credits required. The number of credits was then multiplied by an average credit price to estimate the final estimated compensatory mitigation cost associated with the Lower Little Tallapoosa River 14. Refer to Table 6 contained in the following section entitled "Project Construction Cost Estimate Narrative" for estimated impacts to jurisdictional waters and an estimate of mitigation credits required and associated costs.

Stream Buffer Variance

The Georgia Erosion and Sedimentation Act of 1975 (GESA), as amended, requires that a 25-foot vegetated buffer be maintained along all state waters. Any land disturbing activities within the buffer would require obtaining a stream buffer variance from the EPD. The local issuing authority is responsible for determining if state waters are on-site and is responsible for determining if a stream buffer variance is required.

The GESA has several exemptions including public water system reservoirs. Attendant features such as pipelines and roadways, would be exempt if stream crossings are nearly perpendicular.

EPD Water Withdrawal Permit

Georgia EPD requires a permit for withdrawal of 100,000 gallons per day or more of either surface water or ground water. In addition to justification of need for water for up to 50 years in the future, water withdrawal permits typically require the preparation of water conservation, drought contingency, water supply/watershed protection, and reservoir management plans. A public hearing may be required as part of the withdrawal permitting process. EPD requires that its comments on the component plans be addressed before moving forward with issuing the water withdrawal permit. Based on previous permitting experience, a water withdrawal permit can be obtained within 5 to 7 months, depending on EPD's review time and the extent of their comments.

Source Water Protection Plan

Amendments to the Federal Safe Drinking Water Act (SDWA) have brought about a new approach for ensuring clean and safe drinking water served by public water supplies in the United States. Management of a drinking water source now requires a Source Water Protection Plan. This plan basically defines watershed management strategies for ensuring that the water supply is not compromised by potential pollutant sources. Typically these sources are unmanaged development, but they can also include industrial sources that can potentially contaminate the water supply. The entity that operates this reservoir for water supply would be required to produce and implement the Plan. The Plan should also address any source water from outside the reservoir watershed that would be used to fill the reservoir, i.e., pumped/storage sources. The cost and schedule for producing a Source Water Assessment and the corresponding Source Water Protection Plan have not been included in any of the estimates presented in the report.

PROJECT CONSTRUCTION COST ESTIMATE NARRATIVE

Dam and Reservoir

The construction cost estimate for the proposed dam was based upon the general description provided in the background section of the report. Additionally, the following assumptions were made regarding the geometry of the dam.

- Upstream slope of 3H to 1V
- Downstream slope of 3H to 1V
- Upstream slope wave action protection in the form of riprap from 30 feet below the crest of the dam to 5 feet below the crest of the dam. Riprap supported by a berm located 30 feet below top of dam.
- Downstream slope having nearly horizontal 12-foot wide berms at 30-foot vertical intervals to control surface water runoff and erosion
- Crest of dam having a width of 25-feet

In addition to the above geometric considerations, the following internal drainage configurations were also considered in the estimation of construction costs.

- Chimney drain located at the downstream edge of the crest
- Trench drain located at 1/3 the distance from the downstream toe to the crest

A plan view and cross section of the proposed dam is provided in Figures 10 and 11.

Contained below are the items estimated to develop the construction cost estimate. We caution that the quantities and associated prices are based upon limited engineering evaluation and will likely change as the project proceeds into detailed evaluation and design.

Mobilization and Demobilization

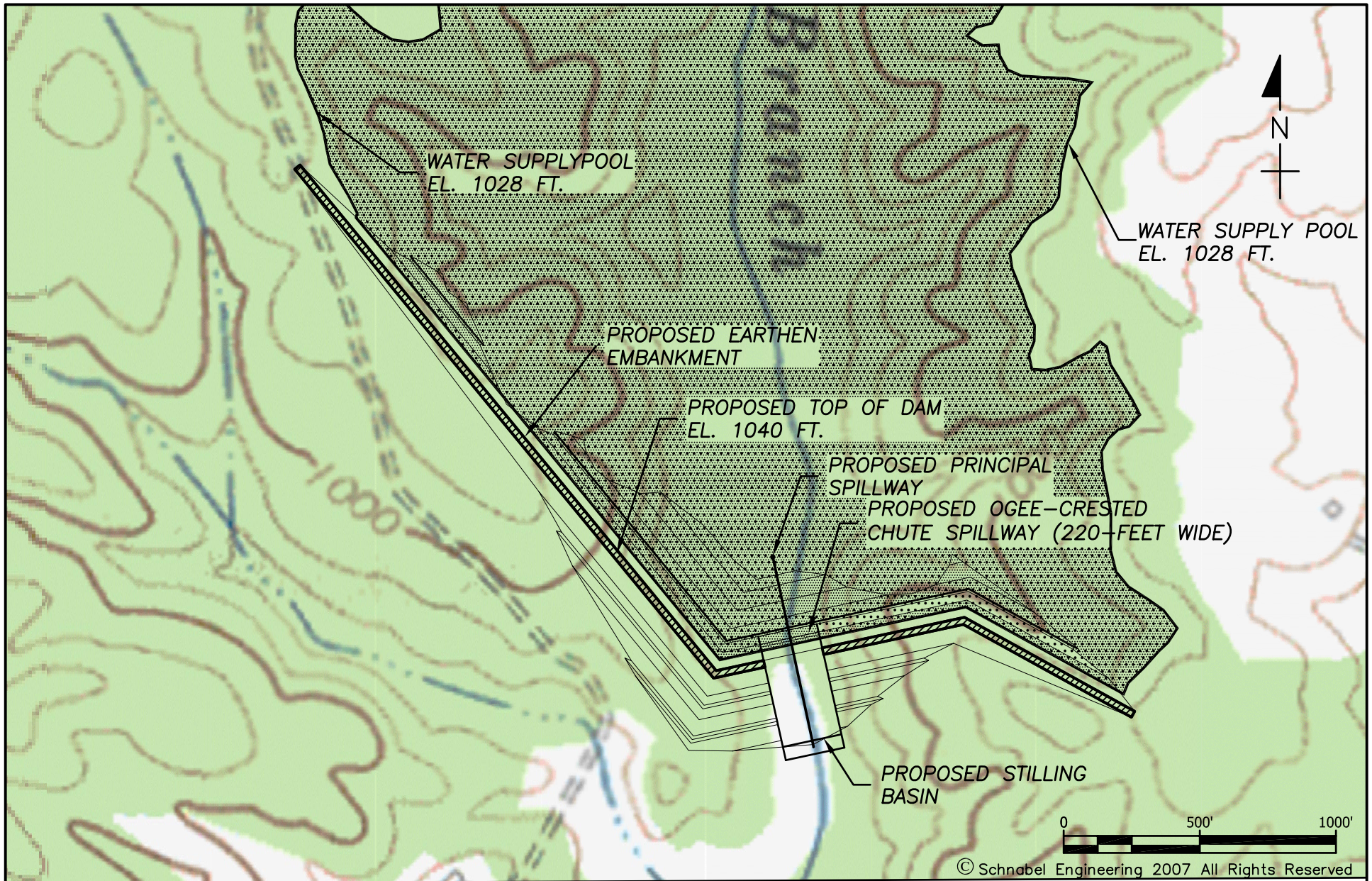
Mobilization and demobilization is a lump sum item estimated at 6 percent of the unit rate sum of the construction items.

Erosion and Sedimentation Control

Erosion and sedimentation control is a lump sum item estimated at 2 percent of the sum of unit rate construction items.

Control of Water

Control of water is a lump sum item estimated at 3 percent of the sum of unit rate construction items. This item includes the control of both surface water and groundwater and will likely consist of stream diversion, cofferdam construction and maintenance, pumping, and well points, as well as any other means of controlling water during construction.



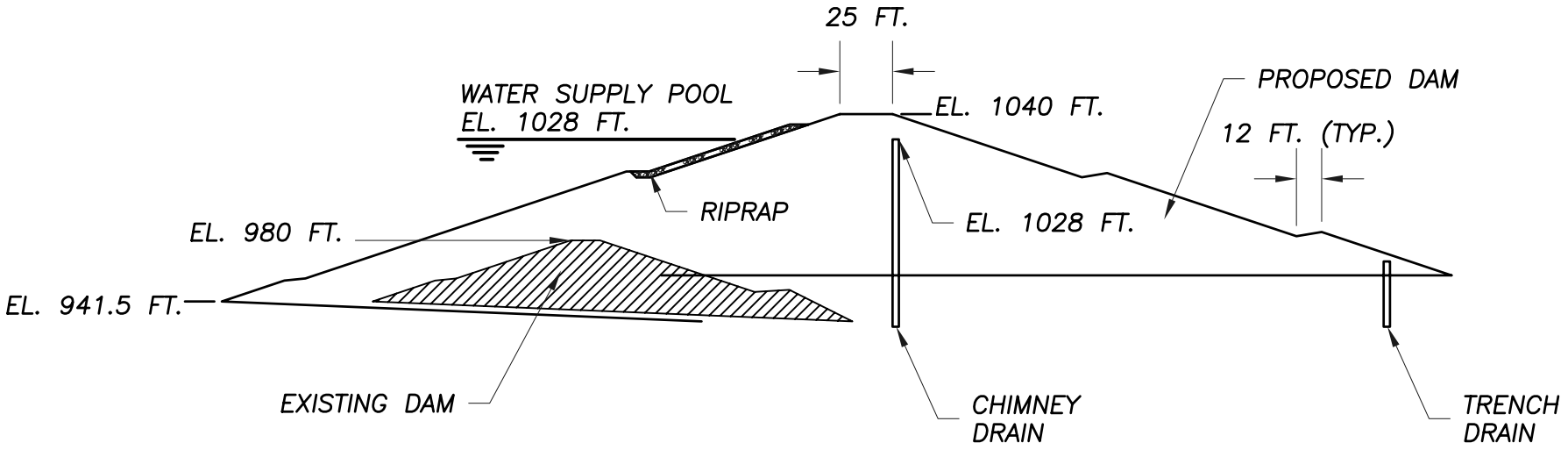
Schnabel Engineering

NRCS WATERSHED DAM ASSESSMENT
LOWER LITTLE TALLAPOOSA NO. 14

PROPOSED PLAN VIEW

PROJECT NO. 07170030.01
FIGURE 10

1/7/2008 17:00 G:\2007 Jobs\07170030.03 assessment JJG\Lower Little Tallapoosa 14\CAD\41-Proposed Site Plan (Figure 10).dwg



0 80' 160'

Schnabel Engineering

NRCS WATERSHED DAM ASSESSMENT
LOWER LITTLE TALLAPOOSA NO. 14

TYPICAL SECTION

PROJECT NO. 07170030.01
FIGURE 11

Clearing

Clearing is a unit rate item measured in acres associated with the removal of trees and other vegetation from the reservoir. The estimated area of clearing was assumed to be equal to the surface area of the reservoir at the normal pool elevation.

Clearing and Grubbing

Clearing and grubbing is a unit rate item measured in acres associated with the removal of trees, other vegetation, and associated root mats in the areas to receive structural fill or concrete. The estimated area of clearing and grubbing was assumed to be equal to the footprint of the proposed dam plus an additional 50-foot perimeter around the proposed dam.

Earth Fill

Earth Fill is a unit rate item measured in cubic yards. The computed volume of earth fill represents the estimated quantity required to construct the dam as described herein. The estimated quantity was computed using an AutoCad Civil 3D computer model based on the proposed grading and existing topography. In addition to the proposed embankment earth fill, foundation excavation backfill was calculated (see Excavation, Common for details) and added to the embankment earth fill to determine the total quantity of earth fill.

Drain Fill

Drain Fill is a unit rate item measured in cubic yards. The computed volume of drain fill represents the estimated quantity of fine and coarse-grained drain material required to construct the internal drainage system as described herein. For the purposes of this study, no differentiation was made between fine and coarse drain fill. In addition, the quantity for the trench drain was assumed to be equal to half of the chimney drain quantity. The chimney drain was assumed to have a top elevation equal to the proposed normal pool elevation and a bottom elevation approximated at the limits of the foundation excavation. The chimney drain was assumed to have a width of three feet and run the length of the dam from one abutment, into the floodplain, and up the other abutment tying into residual soils.

Excavation, Common

Excavation, Common is a unit rate item measured in cubic yards associated with the removal of unsuitable material (soils) within and adjacent to the footprint of the proposed dam. The volume of common excavation was calculated by approximating the surface area of the floodplain within the limits of clearing and grubbing as well as the depth of excavation within the same area. The surface area of the floodplain was approximated using available topographic maps. The depth of excavation was estimated from the boring data included in the design plans for the existing dam.

Riprap

Riprap is a unit rate item measured in tons. The computed weight of riprap represents the estimated quantity required to construct the wave-action berm as described herein. Riprap was assumed to be placed on the upstream slope of the dam. The section of riprap was assumed to extend 30 vertical feet, have a thickness of about 2-¾ feet, and traverse the length of the proposed dam.

Permanent Turf Establishment

Permanent Turf Establishment is a unit rate item measured in acres associated with the establishment of a permanent turf at the conclusion of construction activities for the proposed dam. The estimated area of permanent turf establishment was assumed to be equal to the estimated area of clearing and grubbing.

Concrete, Class 4000

Concrete, Class 4000 is a unit rate item measured in cubic yards associated with the construction of the reinforced concrete auxiliary chute spillway. The volume of concrete was estimated by comparing the proposed auxiliary spillway drop in elevation and width to the drops in elevation and widths of constructed reinforced concrete chute spillways. A relationship was developed between the drop in elevation and width of the constructed spillways and the required quantity of concrete. This relationship was applied to the proposed dam to estimate the quantity of concrete.

Principal Spillway Reinforced Concrete Pressure Pipe

Reinforced Concrete Pressure Pipe (RCPP) is a unit rate item measured in feet. The computed length of RCPP represents the estimated quantity required to construct the principal spillway conduit described herein. The RCPP was assumed to be placed through the base of the proposed dam from the upstream toe to the downstream toe. The diameter of the pipe was assumed to be equal to the diameter of the pipe in the existing dam.

Concrete, Class 3000 (mass)

Concrete, Class 3000 is a unit rate item measured in cubic yards associated with the construction of the concrete cradle beneath the principal spillway pipe. The concrete cradle was assumed to be designed as a Soil Conservation Service Type A2 cradle and run the length of the principal spillway pipe minus ten feet.

Reinforced Concrete Riser

The Reinforced Concrete Riser is a lump sum item associated with the construction of the reinforced concrete principal spillway structure. The cost was estimated by comparing the proposed principal spillway riser height to the heights of constructed reinforced concrete riser structures. A relationship was developed between the height of the

constructed spillways and the cost to construct them. This relationship was utilized to estimate the cost of the proposed riser structure.

Land Acquisition

The costs associated with land acquisitions are unit rate items based upon the number of acres that will need to be purchased at the top-of-dam elevation, the number of acres that will need to be managed for a 150-foot buffer around the normal pool, and the number of houses that will need to be purchased. For the purposes of the buffer management, only the portions of the buffer above top-of-dam elevation were considered. The costs to purchase the land were estimated based upon available records of recent land sales. The cost to manage the buffer was assumed to be 60 percent of the land purchase cost. The cost of each structure impacted was assumed to be $200,000.

Roadway Relocation

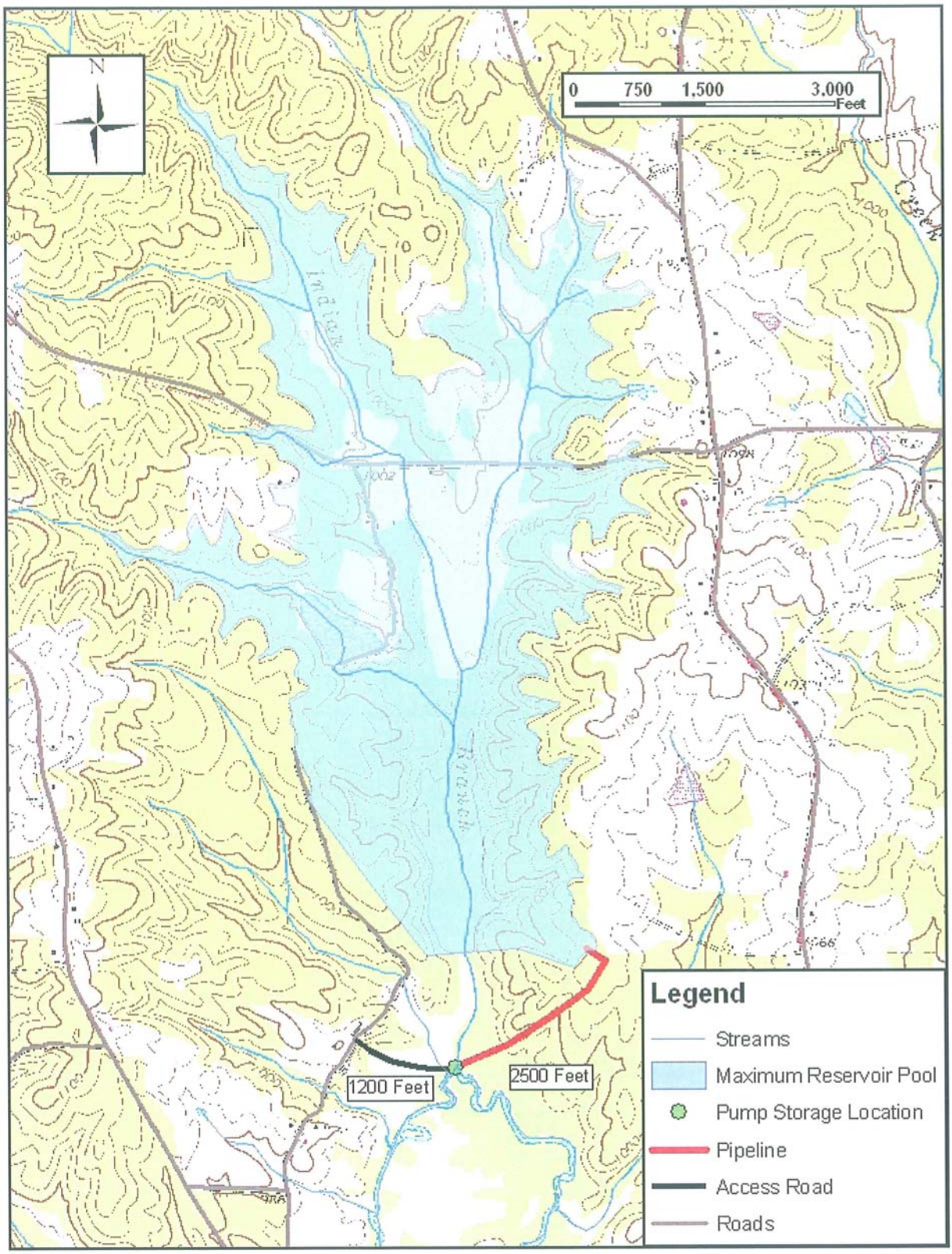
To construct the proposed project, one road will need to be closed. Since the increase in travel distance from either side of the reservoir to the closest town on the opposite of the side of the reservoir was increased by less than 5 miles, no consideration was given to the relocation of the road.

Pump Station and Pipeline Cost Estimation

The pump storage location for Lower Little Tallapoosa River Reservoir 14 is located on Indian Creek at its confluence with Indian Branch. The reservoir is located approximately 0.25 miles upstream on Indian Branch. With a water supply pool elevation of 1028 feet, Reservoir 14 has an average day yield of approximately 7.5 MGD. A 30-inch ductile iron pipeline was selected to carry water from the pump storage location to the reservoir. This pipeline is approximately 0.5 miles in length and will pump water from the storage location elevation of 950 feet to the 1028 feet height of the reservoir water surface. A cascading structure will need to be constructed where the pipe comes into the reservoir to provide aeration and erosion control.

One 10-MGD pump and three 5-MGD pumps were selected at the pump storage location to pump water to the reservoir. This gives a firm pumping capacity of 15-MGD, which is roughly twice the daily yield of the reservoir, the standard assumption for pump capacity. This pumping capacity will allow the reservoir to remain stable in times of peak water demand, as well as give redundancy in the case of failure in one of the pumps. An access road will need to be constructed in order to construct and maintain the pumping station on Indian Creek. This road, shown on Figure 12, will run approximately 0.22 miles from Turner Road. The cost opinion for these components is found in the appendix.

Figure 12
Project Location Map



Compensatory Mitigation

The simplest mitigation option is typically purchasing credits from a bank. Compensatory mitigation credits may be purchased from an approved mitigation bank or through the Georgia Land Trust Service Center if a bank is not available within the project area. Based on recent projects, wetland credits range from $7,000-$10,000 per credit and stream credits range from $70-$110 per credit. An option to purchasing credits is to obtain credits by conducting on-site restoration or preservation of jurisdictional waters.

Table 6
Lower Little Tallapoosa 14 Estimated Impacts and Overall Mitigation Banking Cost Analysis

Impact Type	Estimated Impact Acres/Linear Feet	Projected Credits Needed	Projected Cost* $90/stream credit $7,500/wetland credit
Wetland	32.74 A.	219	$1,642,500
Intermittent Stream	8,748 l.f.	66,485	$5,983,650
Lower Perennial Stream	20,059l.f.	254,749	$22,927,410
Open Water	44.90 A.	256	$1,920,000
Total	**77.64 acres / 28,807 lf**	**475 wetland / 321,234 stream****	**$32,473,560**

*Cost is based on recent quotes from banks within the Upper Tallapoosa Basin. Actual banking price may be higher or lower than estimated depending on the date of purchase and credit availability.**Total required credits calculated using the March 2004 Standard Operating Procedure mitigating guidelines established by the US Army Corps of Engineers, which only serves as a guideline for large projects.

Estimated Project Construction Cost

The total project cost is estimated at $112,000,000. Table A-5, located in the appendix, shows an itemized breakdown of the costs associated with enlarging the existing dam and reservoir. These costs are estimates and are based on multiple assumptions.

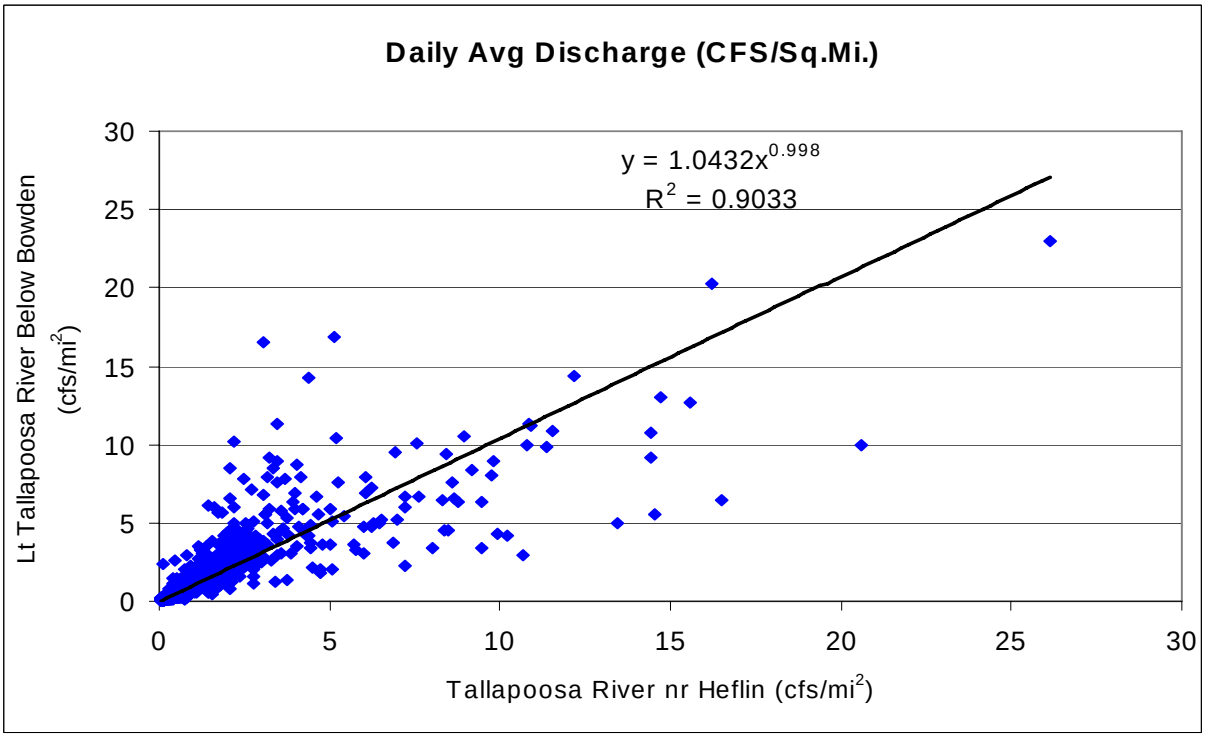
APPENDIX

FIGURES

TABLES

Little Tallapoosa River below Bowden (USGS 02413210) vs
Tallapoosa River near Heflin (USGS 02412000)

Figure A-1



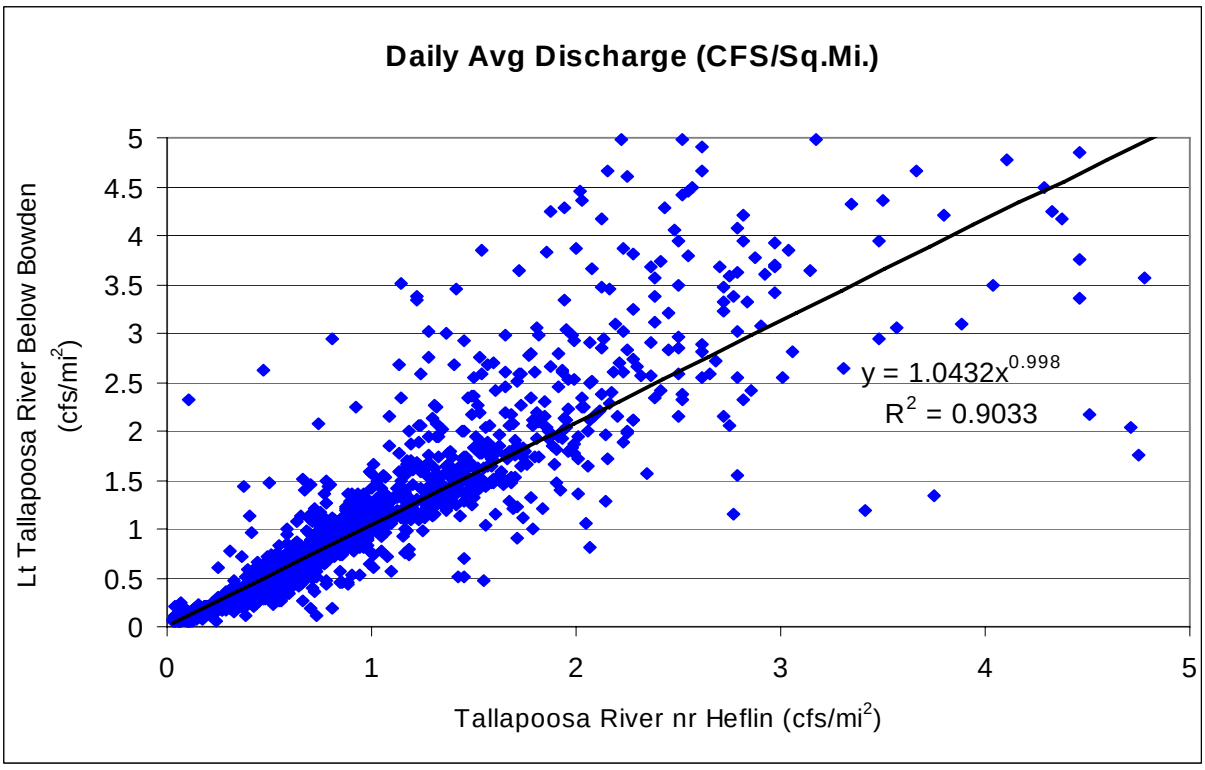
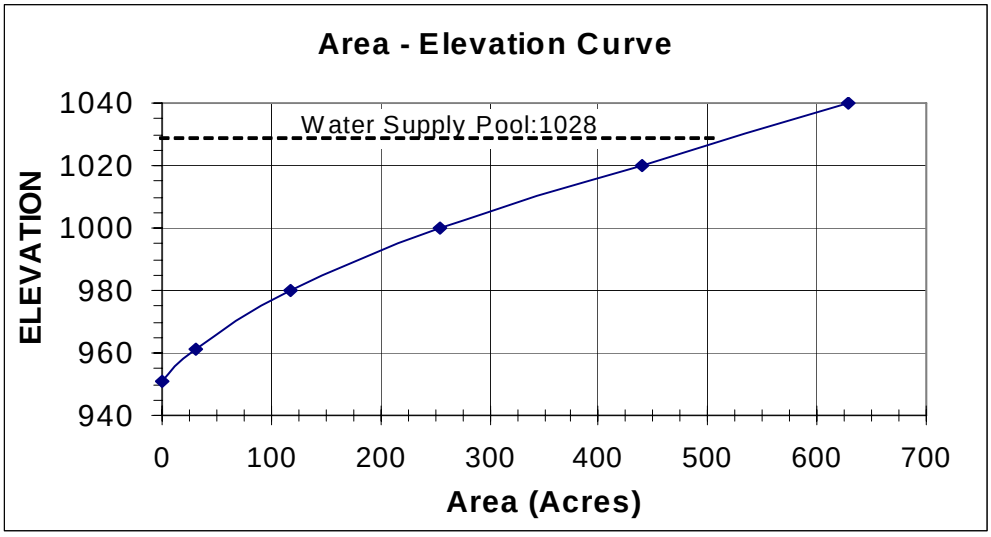


Figure A-2

Lower Little Tallapoosa 14
Area and Storage Curves

Elev.	Area Acres	Area mg/in	Inc. Vol. A-FT	Cumulative Vol A-FT	Cumulative Vol M Gal.
951	0.0	0	0	0	0
961	31.6	1	158	158	51
980	116.8	3	1409	1567	511
1000	254.5	7	3713	5280	1721
1020	439.9	12	6944	12224	3984
1040	628.8	17	10687	22911	7467



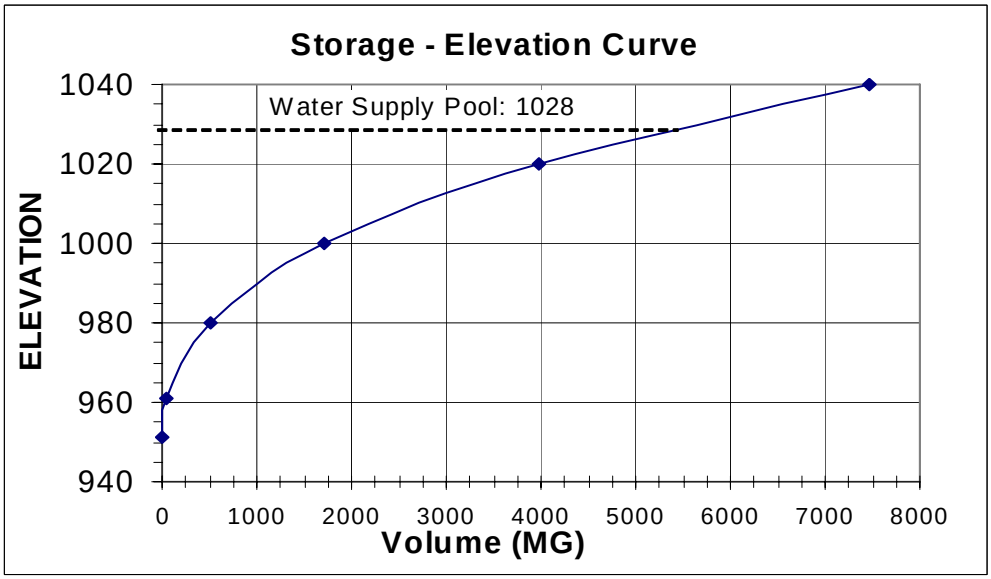
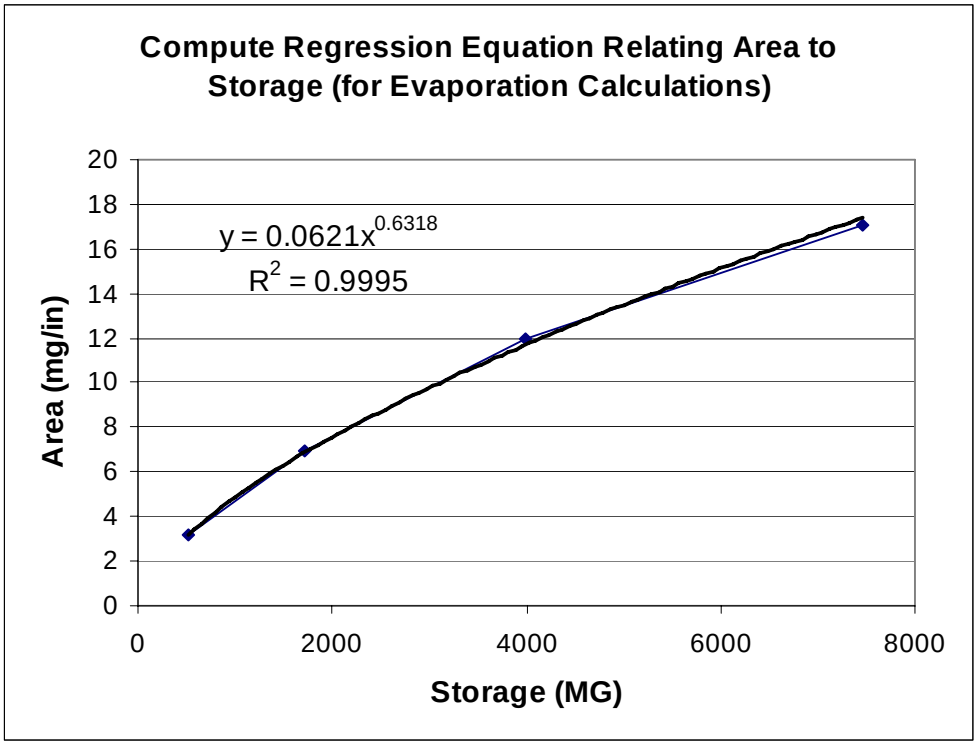
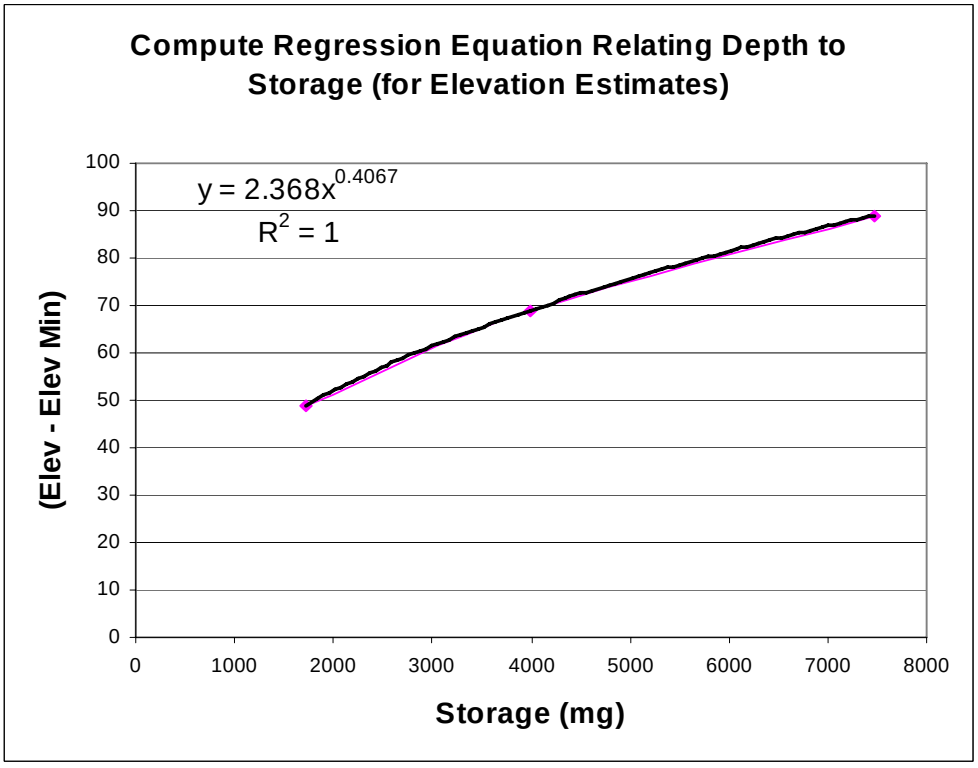


Figure A-3

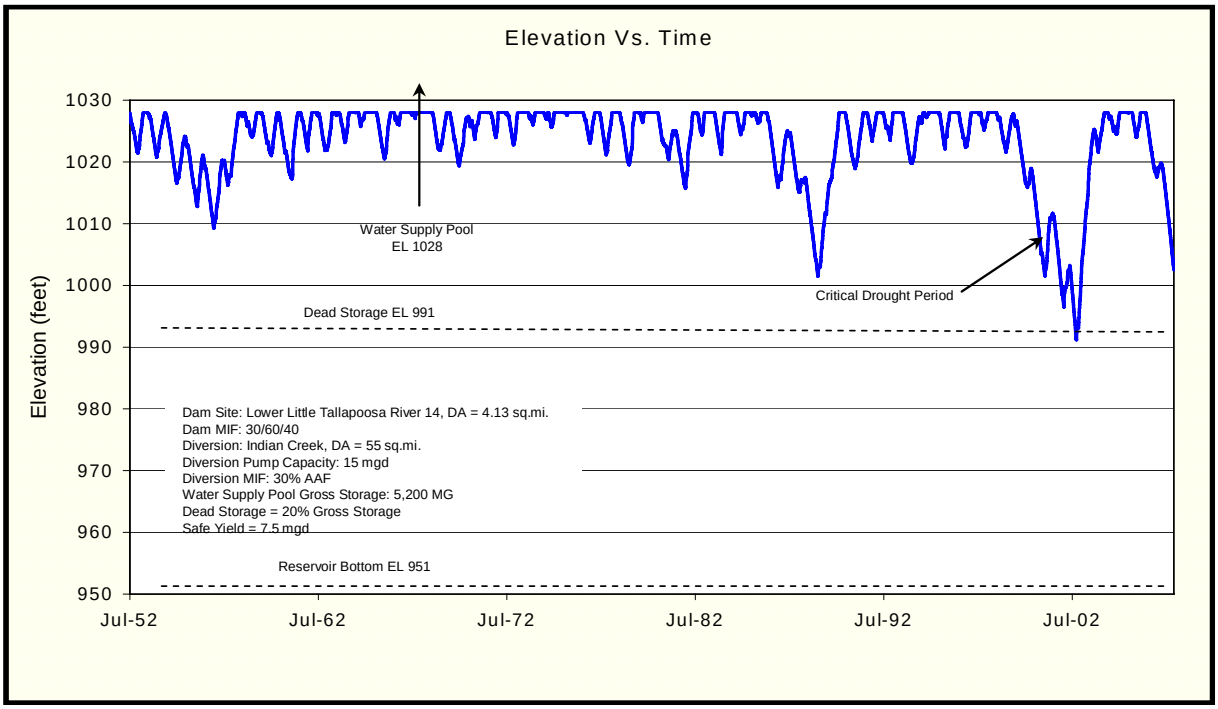
Lower Little Tallapoosa River 14

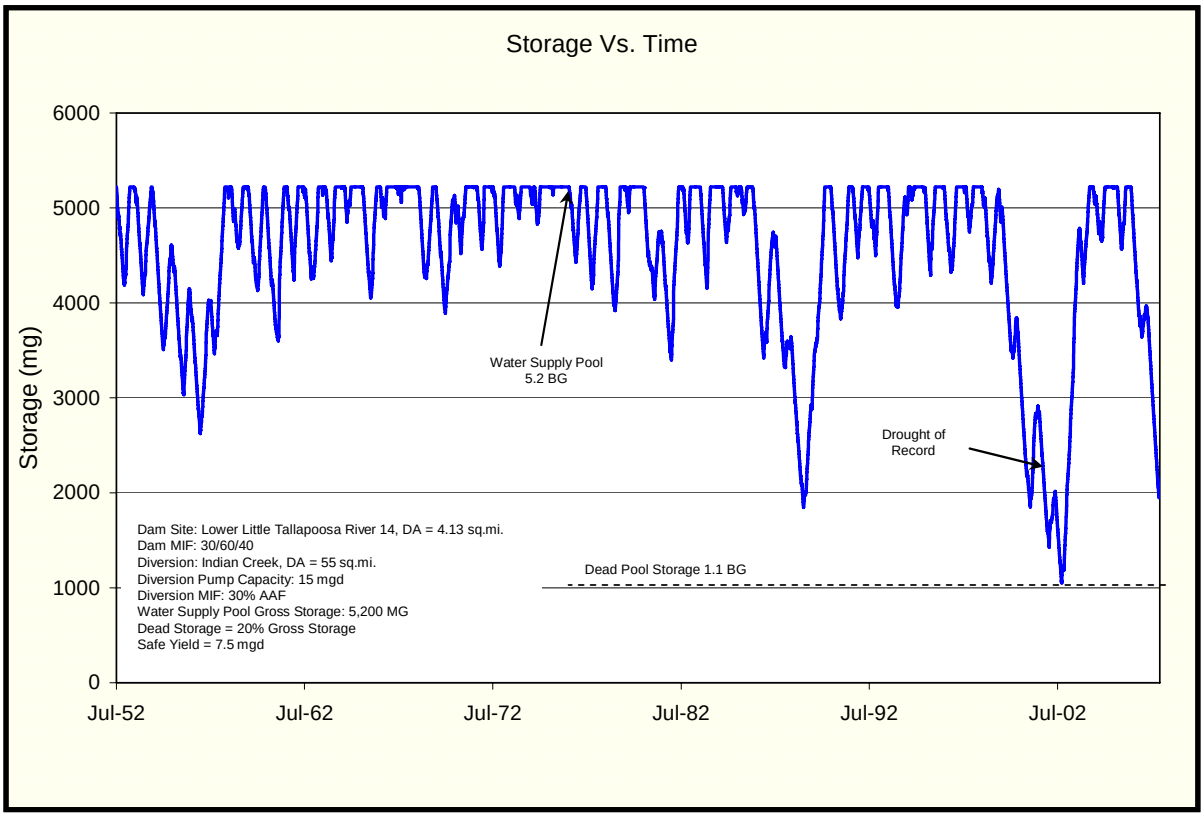




Lower Little Tallapoosa River 14

Figure A-4





WATERSHED DAM ASSESSMENT - LOWER LITTLE TALLAPOOSA RIVER 14

Carroll County, Georgia (7194-001)

OPINION OF PROBABLE CONSTRUCTION COST ESTIMATE - CONCEPTUAL LEVEL

Summary by Division

TABLE A-1

Division	01 - River Intake and Pump Station and Access Road	02 - 30 - inch Raw Water Force Main and Venturi Vault	03 - Reservoir Inlet Structure	TOTAL	% of Total
1	$0.63	$0.05	$0.05	$0.73	9.12%
2	$0.90	$0.00	$0.04	$0.95	11.85%
3	$0.76	$0.02	$0.27	$1.05	13.10%
4	$0.07	$0.00	$0.00	$0.07	0.90%
5	$0.02	$0.00	$0.00	$0.02	0.28%
6	$0.00	$0.00	$0.00	$0.00	0.00%
7	$0.01	$0.00	$0.00	$0.01	0.19%
8	$0.03	$0.00	$0.00	$0.03	0.38%
9	$0.05	$0.00	$0.00	$0.05	0.63%
10	$0.00	$0.00	$0.00	$0.00	0.00%
11	$1.21	$0.00	$0.06	$1.27	15.86%
12	$0.00	$0.00	$0.00	$0.00	0.00%
13	$0.00	$0.00	$0.00	$0.00	0.00%
14	$0.09	$0.00	$0.00	$0.09	1.15%
15	$0.41	$0.39	$0.01	$0.82	10.21%
16	$0.93	$0.06	$0.00	$0.98	12.28%
17	$0.22	$0.02	$0.00	$0.24	3.00%
Structure Contingency	$0.53	$0.00	$0.00	$0.53	6.69%
Markup	$0.97	$0.09	$0.09	$1.15	14.37%
Structure Total (without Contingency)	$6.84	$0.61	$0.53	$7.98	100.00%
Project Contingency	**$2.05**	**$0.18**	**$0.16**	**$2.39**	**30.00%**
Structure Total (with Contingency)	**$8.89**	**$0.80**	**$0.68**		

*All Figures are in Millions

TOTAL $10.38 *

LOWER LITTLE TALLAPOOSA RIVER 14:

Maximum Safe Reservoir Yield: 7.5 MGD

RWPS Firm Pumping Capacity: 15.0 MGD

RWFM Pipe Diameter: 30-inches

WATERSHED DAM ASSESSMENT - LOWER LITTLE TALLAPOOSA RIVER 14
Carroll Georgia (7194-001)

01
DECEMBER 2007
TABLE A-2

OPINION OF PROBABLE CONSTRUCTION COST - CONCEPTUAL LEVEL

01 - River Intake and PS

No.	Spec. Sect.	Description	Unit	Qty	Labor $$		Material $$		Equipment $$		Subcontractor $$		Total
					Unit	Total	Unit	Total	Unit	Total	Unit	Total	
01 - Lower Little Tallapoosa River 14: River Intake and Pump					**4 - Channel Intake Pump Station**				**Pump Station Firm Capacity is 15MGD**				
		Div 1											
1	1000	General Conditions	LS	1		$226,000		$179,000		$226,100		$0	$631,100
		Div 2											
2	2200	Earth Work	LS	1	$34,050.00	$34,050	$6,250.00	$6,250	$40,362.00	$40,360	$299,600.00	$299,600	$380,260
3		Access Road	LF	1200		$0		$0		$0	$110.00	$132,000	$132,000
4		Creek Crossing	EA	2		$0		$0		$0	$50,000.00	$100,000	$100,000
5	2831	10' Galv. Chain Link Fence	LF	3000		$0		$0		$0	$30.00	$90,000	$90,000
6	2831	Dewatering / Pre-Excavation Preparation	LS	1	$50,000.00	$50,000	$20,000.00	$20,000	$100,000.00	$100,000	$30,000.00	$30,000	$200,000
		Div 3											
7	3250	Water Stop	LF	500	$1.25	$630	$2.00	$1,000		$0		$0	$1,630
8	3300	Concrete Bridge	SF		$2.00	$0		$0	$3.50	$0	$20.00	$0	$0
9	3300	Concrete	LS	1	$240,987.00	$240,990	$444,318.00	$444,320	$72,930.00	$72,930	$0.00	$0	$758,240
		Div 4											
10	4210	Brick Veneer	SF	3300		$0		$0		$0	$14.50	$47,850	$47,850
11	4220	Concrete Masonry Unit - Reinforced	SF	3300		$0		$0		$0	$7.25	$23,930	$23,930
		Div 5											
10	5524	Aluminum Handrail	LF	200	$6.00	$1,200	$35.00	$7,000	$2.90	$580		$0	$8,780
11		Ladder	VF	20	$50.00	$1,000	$150.00	$3,000	$15.00	$300		$0	$4,300
12	5530	Aluminum Grating Landing	SF	32	$10.00	$320	$45.00	$1,440	$10.00	$320		$0	$2,080
13	5530	Aluminum Grating	SF	240	$10.00	$2,400	$20.00	$4,800		$0		$0	$7,200
		Div 6											
		Div 7											
14		Membrane Roofing	SF	1500		$0		$0		$0	$5.00	$7,500	$7,500
15		Dampproofing - Walls	SF	3300		$0		$0		$0	$0.56	$1,850	$1,850
16		1" Rigid Insulation - Walls	SF	3300		$0		$0		$0	$1.07	$3,530	$3,530
17	7210	Walls - Core Fill Foam Insulation (12" CMU)	SF	3300		$0		$0		$0	$0.61	$2,010	$2,010
		Div 8											
18	8120	Hollow Metal Doors, Hardware, and Frames - Single	EA	10	$150.00	$1,500	$400.00	$4,000		$0		$0	$5,500
19	8120	Hollow Metal Doors, Hardware, and Frames - Double	EA	2	$150.00	$300	$800.00	$1,600		$0		$0	$1,900
20		Windows	LS	1	$3,000.00	$3,000	$8,000.00	$8,000	$1,000.00	$1,000		$0	$12,000
21	8331	Roll Up Aluminum Door (10'x12')	EA	2	$800.00	$1,600	$4,500.00	$9,000	$50.00	$100		$0	$10,700
		Div 9											
22	9900	Painting	LS	1		$0		$0		$0	$50,000.00	$50,000	$50,000
		Div 10											
		Div 11											
23		Screens	EA	4	$3,500.00	$14,000	$200,000.00	$800,000	$500.00	$2,000		$0	$816,000
24		Eductors	EA	20	$200.00	$4,000	$2,500.00	$50,000	$50.00	$1,000		$0	$55,000
25		Pumps (10 MGD, 90 Feet Static Head)	EA	1	$3,500.00	$3,500	$140,000.00	$140,000	$1,000.00	$1,000		$0	$144,500
26		Pumps (5 MGD, 90 Feet Static Head)	EA	3	$3,500.00	$10,500	$60,000.00	$180,000	$1,000.00	$3,000		$0	$193,500
		Div 12											
		Div 13											
		Div 14											
27		Bridge Crane	LS	1	$5,000.00	$5,000	$85,000.00	$85,000	$1,500.00	$1,500		$0	$91,500
		Div 15											
28	15062	Ductile Iron Pipe	LS	1	$11,195.00	$11,200	$197,359.83	$197,360	$2,840.00	$2,840	$0.00	$0	$211,400
29		PVC Piping	LS	1	$1,250.00	$1,250	$8,000.00	$8,000	$750.00	$750		$0	$10,000
30		Valves	LS	1	$10,000.00	$10,000	$100,000.00	$100,000	$2,000.00	$2,000	$0.00	$0	$112,000
31		HVAC and Plumbing	LS	1		$0		$0		$0	$80,000.00	$80,000	$80,000
		Div 16											
32	16000	Electrical	LS	1		$0		$0		$0	$550,000.00	$550,000	$550,000
33		CCTV Allowance	LS	0		$0		$0		$0		$0	$0
34		Ductbank	LF	2500		$0		$0		$0	$150.00	$375,000	$375,000
		Div 17											
35	17000	Instrumentation	LS	1		$0		$0		$0	$220,000.00	$220,000	$220,000
		Contingency	LS	10%		$62,000		$225,000		$46,000		$201,000	$534,000
		Subtotals				$684,440		$2,474,770		$501,780		$2,214,270	$5,875,260

Sales Tax @	**7.0%**	$173,200
Labor Burden @	**30.0%**	$205,300
Bonds On Subs @	**1.5%**	$33,200
Subtotal		$6,286,960
Fee @	**7.0%**	$440,100
Insurance & Bonds @	**1.7%**	$114,400
Estimated Construction Cost		**$6,841,460**

Assumptions:

Assumes that EPD will allow withdrawal from the Lower Little Tallapoosa River 14
15 foot wide Asphalt access road with 10-foot high fence
Pump Station firm capacity is 15MGD
Pump Station has a 4 channel intake
Pump Station footprint is approximately 100 feet by 52 feet
Pump Station main building footprint is approximately 35 feet by 48 feet
Pump Station main building also houses the electrical room and is made of brick and block
A Transformer is being provided by the Utility Company at the access road entrance
Estimate DOES NOT include easements acquisitions, land acquisitions, withdrawal permits or mitigations required to build the pump station

WATERSHED DAM ASSESSMENT - (7194-001)

OPINION OF PROBABLE CONSTRUCTION COST ESTIMATE - CONCEPTUAL LEVEL

02 - 30-inch Raw Water Line

02
DECEMBER 2007
TABLE A-3

No.	Spec. Sect.	Description	Unit	Qty	Labor $$		Material $$		Equipment $$		Subcontractor $$		Total
					Unit	Total	Unit	Total	Unit	Total	Unit	Total	
02 - 30-inch Raw Water Line with Venturi Vault													
		Div 1											
1	1000	General Conditions	LS	1		$17,000		$12,000		$16,600		$0	$45,600
		Div 2											
2	2125	Erosion and Sedimentation Control Maintenance - with Unit Bid	MTH			$0		$0		$0		$0	$0
3		Dewatering	LS			$0		$0		$0		$0	$0
4	2510	Asphalt Concrete Pavement - with Unit Bid	LS			$0		$0		$0		$0	$0
5	2523	Concrete Sidewalk and Curbs - with Unit Bid	LS			$0		$0		$0		$0	$0
		Div 3											
6	3300	Miscellaneous Concrete (Venturi Vault)	LS	1	$1,500.00	$1,500	$12,500.00	$12,500	$1,000.00	$1,000	$0.00	$0	$15,000
		Div 4											
		Div 5											
		Div 6											
		Div 7											
		Div 8											
		Div 9											
		Div 10											
		Div 11											
		Div 12											
		Div 13											
		Div 14											
		Div 15											
7		**30" DIP**	**Depth**	**7**		**Depth of Cover**	**4**						
8		30" Pipe Excavation - Earth (compacted volume)	CY	2483	$0.75	$1,862		$0	$3.00	$7,448		$0	$9,310
9		30" Pipe Excavation - Trench Rock (compacted volume)	CY	828		$0		$0		$0	$35.00	$28,964	$28,964
10		Trench Box	LF	2500		$0	$1.00	$2,500		$0		$0	$2,500
11		30" DIP Pressure Class 200	LF	2050	$7.67	$15,715	$80.26	$164,525	$2.50	$5,125		$0	$185,365
12		30" Pipe Bedding (compacted volume)	CY	509	$1.00	$509	$13.00	$6,620	$1.00	$509		$0	$7,639
13		30" Pipe Backfill (compacted volume)	CY	2346	$1.00	$2,346		$0	$4.00	$9,386		$0	$11,732
14		Import Backfill Materials (loose volume, assume 10% swell)	CY	150		$0	$13.00	$1,948		$0		$0	$1,948
15		Haul off Rock (assume 15% swell) - with Unit Bid	CY	952		$0		$0		$0	$15.00	$14,275	$14,275
16		30" 90-degree Bend	EA	1	$190.00	$190	$5,340.60	$5,341	$50.00	$50		$0	$5,581
17		30" 45-degree Bend	EA		$190.00	$0	$4,181.40	$0	$50.00	$0		$0	$0
18		30" 22.5-degree Bend	EA		$190.00	$0	$3,709.44	$0	$50.00	$0		$0	$0
19		30" 11.25-degree Bend	EA	2	$190.00	$380	$3,436.20	$6,872	$50.00	$100		$0	$7,352
20		30" DIP Pressure Class 200 RJ	LF	450	$9.17	$4,125	$109.82	$49,421	$2.50	$1,125		$0	$54,671
21													
22		Earthwork Calculations				$0		$0		$0		$0	$0
23		Pipe Excavation - Total Compacted Volume	CY	3310		$0		$0		$0		$0	$0
24		Rock - Total Compacted Volume (assume 25% of excavation)	CY	828		$0		$0		$0	$37.00	$30,619	$30,619
25		Pipe Bedding - Total Compacted Volume	CY	509		$0		$0		$0		$0	$0
26		Pipe Backfill - Total Compacted Volume Needed	CY	2346		$0		$0		$0		$0	$0
27		On-Site Backfill Material Available - Compacted Volume	CY	2483		$0		$0		$0		$0	$0
28		Materials for Disposal - Compacted Volume	CY	136	$5.00	$681		$0	$5.00	$681		$0	$1,362
29													
30		Air Release Valve and Manhole (3 each)	LS	1	$2,200.00	$2,200	$26,400.00	$26,400	$1,800.00	$1,800	$0.00	$0	$30,400
31													
		Div 16											
32	16000	Electrical	LS	1		$0		$0		$0	$55,000.00	$55,000	$55,000
		Div 17											
33	17000	Venturi Meter	LS	1	$1,000.00	$1,000	$10,500.00	$10,500	$500.00	$500		$0	$12,000
34	17000	Instrumentation	LS	1		$0		$0		$0	$7,500.00	$7,500	$7,500
		Contingency	LS	0%		$0		$0		$0		$0	$0
		Subtotals				$47,509		$298,627		$44,324		$136,359	$526,818

Sales Tax @	**7.0%**	$20,900
Labor Burden @	**30.0%**	$14,300
Bonds On Subs @	**1.5%**	$2,000
Subtotal		$564,018
Fee @	**7.0%**	$39,500
Insurance & Bonds @	**1.7%**	$10,300
Estimated Construction Cost		**$613,818**

Assumptions:

Estimate DOES NOT include easements acquisitions, land acquisitions or mitigations required to build the pump station

Assumed 25% of the excavated material is rock

$246 per LF

WATERSHED DAM ASSESSMENT - (7194-001)

OPINION OF PROBABLE CONSTRUCTION COST ESTIMATE - CONCEPTUAL LEVEL

03 - Reservoir Inlet Structure

TABLE A-4

No.	Spec. Sect.	Description	Unit	Qty	Labor $$		Material $$		Equipment $$		Subcontractor $$		Total
					Unit	Total	Unit	Total	Unit	Total	Unit	Total	
03 - Reservoir Inlet Structure													
		Div 1											
1	1000	General Conditions	LS	1		$18,000		$14,500		$18,300		$0	$50,800
		Div 2											
2	2200	Earth Work	LS	1	$5,000.00	$5,000	$2,639.00	$2,640	$4,926.00	$4,930	$31,300.00	$31,300	$43,870
		Div 3											
3	3250	Water Stop	LF	500	$1.25	$630	$2.00	$1,000		$0		$0	$1,630
4	3300	Concrete	LS	1	$82,952.00	$82,950	$159,839.00	$159,840	$26,200.00	$26,200	$0.00	$0	$268,990
		Div 4											
		Div 5											
7	5524	Aluminum Handrail	LF		$6.00	$0	$35.00	$0	$2.90	$0		$0	$0
8		Ladder	VF		$50.00	$0	$150.00	$0	$15.00	$0		$0	$0
9	5530	Aluminum Grating Landing	SF		$10.00	$0	$45.00	$0	$10.00	$0		$0	$0
10	5530	Aluminum Grating	SF		$10.00	$0	$20.00	$0		$0		$0	$0
		Div 6											
		Div 7											
		Div 8											
		Div 9											
10	9900	Painting	LS			$0		$0		$0		$0	$0
		Div 10											
		Div 11											
11		Sluice Gates and Operators	EA	2	$2,500.00	$5,000	$25,000.00	$50,000	$1,000.00	$2,000		$0	$57,000
		Div 12											
		Div 13											
		Div 14											
		Div 15											
12	15062	Ductile Iron Pipe	LS	1	$1,000.00	$1,000	$8,500.00	$8,500	$500.00	$500		$0	$10,000
		Div 16											
13	16000	Electrical	LS			$0		$0		$0	$70,000.00	$0	$0
		Div 17											
14	17000	Instrumentation	LS			$0		$0		$0	$25,000.00	$0	$0
		Contingency	LS	0%		$0		$0		$0		$0	$0
		Subtotals				$112,580		$236,480		$51,930		$31,300	$432,290

Sales Tax @	**7.0%**	$16,600
Labor Burden @	**30.0%**	$33,800
Bonds On Subs @	**1.5%**	$500
Subtotal		$483,190
Fee @	**7.0%**	$33,800
Insurance & Bonds @	**1.7%**	$8,800
Estimated Construction Cost		**$525,790**

Table A-5

Lower Little Tallapoosa River Dam No. 14

TOTAL PROJECT OPINION OF COST

Item . No.	Description of Work	Estimated Quantity	Unit	Unit Price	Amount
1.	Mobilization and Demobilization	1	Job	Lump Sum	$1,214,541
2.	Erosion & Sediment Control	1	Job	Lump Sum	$404,847
3.	Control of Water	1	Job	Lump Sum	$607,270
4.	Clearing	526	Ac	$2,500	$1,315,000
5.	Clearing & Grubbing	37	Ac	$5,000	$185,000
6.	Earth Fill	1,127,183	Cu-Yd	$4	$4,508,732
7.	Drain Fill	28,612	Cu-Yd	$75	$2,145,900
8.	Excavation, Common	77,863	Cu-Yd	$5	$389,315
9.	Riprap	38,178	Ton	$75	$2,863,350
10.	Permanent Turf Establishment	37	Ac	$2,000	$74,000
11.	Concrete, Class 4000 (reinforced)	9,681	Cu-Yd	$850	$8,228,850
12.	Concrete, Class 3000 (mass)	133	Cu-Yd	$400	$53,200
13.	30-Inch RCP	710	Feet	$400	$284,000
14.	Principal Spillway Riser	1	Lump Sum	$195,000	$195,000
	Dam Construction Cost Estimate				**$22,469,005**
15.	30-Inch Pipeline	1	Lump Sum	$530,000	$530,000
16.	Cascading Structure	1	Lump Sum	$610,000	$610,000

17.	Pumping Station (Including Raw Water Pumps and Access Road)	1	Lump Sum	$6,840,000	$6,840,000
	Pump Station and Pipeline Cost Estimate				**$7,980,000**
18.	Land Acquisition	629	Ac	$20,000	$12,580,000
19.	Easement Acquisition	57	Ac	$12,000	$686,400
20.	Building Acquisition	18	Structures	$200,000	$3,600,000
	Land Acquisition Cost Estimate				**$16,866,400**
21.	Wetland	219	Credits	$7,500	$1,642,500
22.	Intermittent Stream	66,485	Credits	$90	$5,983,650
23.	Lower Perennial Stream	254,749	Credits	$90	$22,927,410
24.	Open Water	256	Credits	$7,500	$1,920,000
	Impacts and Overall Mitigation Cost Estimate				**$32,473,560**
	Construction, Land Acquisition, Mitigation Estimate				**$79,788,965**
	Contingency at 25%				**$19,947,241**
	Professional Services at 15% *				**$11,968,345**
	Total Project Estimate				**$111,704,551**
	Suggested Project Estimate				**$112,000,000**

*Professional services include but are not limited to engineering, construction management legal, appraisals, and environmental consulting.